#### AFRL-WS-WP-TM-1999-9009

#### AN INVESTIGATION OF THE COMPRESSIVE STRENGTH PROPERTIES OF STAINLESS STEEL SHEET-STRINGER COMBINATIONS



Air Service Information Circular, Volume VII, No. 697

E.H. SCHWARTZ C.B. BROWN

Air Service Engineering Division McCook Field Dayton OH 45430

November 30, 1934

Approved for public release; Distribution unlimited.

DMC QUALITY INSPECTED A

AIR FORCE RESEARCH LABORATORY
AIR FORCE MATERIEL COMMAND
WRIGHT-PATTERSON AIR FORCE BASE OH 45433

#### **REPORT DOCUMENTATION PAGE**

Form Approved OMB No. 0704-0188

Public reporting burden for this collection of Information is estimated to average 1 hour per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. Send comments regarding this burden estimate or any other aspect of this collection of information, including suggestions for reducing this burden, to Washington Headquarters Services, Directorate for Information Operations and Reports, 1215 Jefferson Davis Highway, Suite 1204, Alignoton, VA 22202-4302, and to the Office of Management and Budget, Papersyork, Reduction Project (0704-0188), Washington, DC 2050-6.

collection of Information, including suggestions Davis Highway, Suite 1204, Arlington, VA 222			quarters Services, Directorate for d Budget, Paperwork Reduction (	r Information Project (0704	Operations and Reports, 1215 Jefferson -0188), Washington, DC 20503.				
1. AGENCY USE ONLY (Leave blank)   2. REPORT DATE   3. REPORT TYPE AND DATES COVERED					• • •				
	November 30,1934 FINAL S								
4. TITLE AND SUBTITLE	COL	ADDECCIVE CTDENCTH	DODEDTIES OF	5. FUND	ING NUMBERS				
AN INVESTIGATION OF THE			ROPERTIES OF						
STAINLESS STEEL SHEET-ST Air Service Information Circular									
6. AUTHOR(S)	, ۷0	diffe vii, No. 097							
E.H. SCHWARTZ									
C.B. BROWN									
7. PERFORMING ORGANIZATION	NAM	E(S) AND ADDRESS(ES)			ORMING ORGANIZATION RT NUMBER				
Air Service				REPU	NI NOMBEN				
Engineering Division									
McCook Field									
Dayton OH 45430									
9. SPONSORING/MONITORING AC	<u> </u>	Y NAME(S) AND ADDRESS(ES	3)	10. SPO	NSORING/MONITORING				
Air Service				AGE	NCY REPORT NUMBER				
Engineering Division					* **** **** *** * 4000 0000				
McCook Field					L-WS-WP-TM-1999-9009				
Dayton OH 45430									
44 CUIDDI PAPATADV MOTEC									
11. SUPPLEMENTARY NOTES									
12a. DISTRIBUTION AVAILABILITY	STA	TEMENT		12b. DIS	TRIBUTION CODE				
Approved for public release; Di	stribı	ution unlimited.							
13. ABSTRACT (Maximum 200 wo	rds)								
Historical test whose test data in		es that: 1. The strength pro-	nerties of sheet stringe	r combin	ations are governed by the				
strength properties of the stringe									
highest structural efficiency from				_	•				
the sking thickness to a minimum									
units provided, and is independen	nt of	the stiffener pitch. 3. The	effective width of sheet	t working	with a stiffener at any				
stiffener stress, up to the stiffene		<del>-</del>		_					
provided in the report.				•	•				
•									
14. SUBJECT TERMS	CT	AINH EGG OTTEN TENIOH	E CTDENICTH		15. NUMBER OF PAGES				
STRESS STRAIN RELATIONS		-	•		84				
COMPRESSIVE STRENGTH,	IESI	AND EVALUATION, ST	RAIN(MECHANICS)	),	16. PRICE CODE				
SHEET METAL  17. SECURITY CLASSIFICATION	18. S	SECURITY CLASSIFICATION T	19. SECURITY CLASSIF	ICATION	20. LIMITATION OF ABSTRACT				
OF REPORT		OF THIS PAGE	OF ABSTRACT						
UNCLASSIFIED		UNCLASSIFIED	UNCLASSIFIE	ED	SAR				

### AIR CORPS INFORMATION CIRCULAR

PUBLISHED BY THE CHIEF OF THE AIR CORPS, WASHINGTON, D. C.

Vol. VII

November 30, 1934

No. 697

# AN INVESTIGATION OF THE COMPRESSIVE STRENGTH PROPERTIES OF STAINLESS STEEL SHEET-STRINGER COMBINATIONS

 $\nabla$ 

(AIRCRAFT BRANCH REPORT)



UNITED STATES
GOVERNMENT PRINTING OFFICE
WASHINGTON: 1935

#### TABLE OF CONTENTS

	Page
Summary	1
History	1
Dates and place of tests	1
Object	2
Description	2
Test specimens	2
Description of apparatus	2
Discussion	3
A series tests	3
C series tests	4
Fixity effects	5
Effective radius of gyration	5
Constant $\sigma_{*$	5
Material	6
Application to other stiffeners	6
Comparison with aluminum alloy sheet-stringer tests	6
Application to box sections	6
Variation of stiffener cross section	7 8
Open section stiffener tests	8
Formation of wrinkles	8
Types of failure	9
Weld failures	9
I series corrugations	10
Suggestions for future work	10
References	10
Tables	32
Illustrations	64
Photographs	0.1
LIST OF TABLES	
MAD (1 1122-0	
· · · · · · · · · · · · · · · · · · ·	
Table I. Physical properties of test specimen material	10
II. Computation of stiffener failing stress	11
III Computation of effective width of sheet working with stiffeners of A series	12
IV Column tests of individual stiffeners of 0.029 thickness	13
V Section properties of 0.030 stiffener of A section	14
VI Computations of slenderness of working units of sheet and stiffener	14
VII. Comparison of strength properties of corrugated sheets of 24ST aluminum alloy and stainless	
steel	14
VIII. Compression tests of A series specimens	15
IX Compression tests of B series specimens	17
X Compression tests of C series specimens	18
XI Compression tests of D series specimens	21
XII Compression tests of EA series specimens	21
XIII Compression tests of EB series specimens	25
XIV Compression tests of EC series specimens.	27
XV. Compression tests of F series specimens	30
YVI Compression tests of I series specimens	30

#### LIST OF ILLUSTRATIONS

_		
FIGURE	1.	Typical stiffener section of A, B, C series  Typical stiffener section of EA, EB, EC, and F series
		Typical section of corrugation with flat sheet attached.
		Simple box beam section Simple box beam section
		A Series Curves (L=9.25 Inches)
	5.	Failing load versus number of stiffeners and sheet thickness for various stiffeners
		Failing load versus number of stiffeners and sheet thickness for various stiffeners
		Failing load versus number of stiffeners for 0.019 stiffener
		Failing stress versus stiffener pitch for various stiffener thicknesses with 0.009 sheet attached
		Failing stress versus stiffener pitch for various stiffener thicknesses with 0.014 sheet attached
		Failing stress versus stiffener pitch for various stiffener thicknesses with 0.019 sheet attached
		Failing stress versus stiffener pitch for various stiffener thicknesses with 0.029 sheet attached
	12.	Failing stress versus stiffener pitch for various stiffener thicknesses with 0.049 sheet attached.
	13.	Failing stress versus stiffener pitch for 0.009 stiffeners with various sheet attached
	14.	Failing stress versus stiffener pitch for 0.014 stiffeners with various sheet attached
	15.	Failing stress versus stiffener pitch for 0.019 stiffeners with various sheet attached
		Failing stress versus stiffener pitch for 0.029 stiffeners with various sheet attached
		Failing stress versus stiffener pitch for 0.049 stiffeners with various sheet attached.
		Failing stress versus percent reinforcement for 0.009 stiffeners with various sheet attached.
		Failing stress versus percent reinforcement for 0.014 stiffeners with various sheet attached.
		Failing stress versus percent reinforcement for 0.019 stiffeners with various sheet attached.
		Failing stress versus percent reinforcement for 0.029 stiffeners with various sheet attached.
	22.	Failing stress versus percent reinforcement for 0.049 stiffeners with various sheet attached.
,		B Series Curves (L=9.25 Inches)
	23	Failing load versus sheet thickness for various stiffener thickness.
		Failing stress versus stiffener thickness for various sheet thicknesses
		C Series Curves
	25.	σ, versus stiffener thickness for various lengths
	<b>26</b> .	C versus sheet thickness
	<b>27</b> .	Average failing load per stiffener versus length for 0.019 stiffener with various sheet
	<b>2</b> 8.	Average failing load per stiffener versus length for 0.029 stiffener with various sheet
		Failing load versus length for various pitch with 0.029 stiffener with 0.029 sheet
		Failing load versus length for various pitch with 0.029 stiffener with various sheet.
		Failing load versus length for various pitch with 0.019 stiffener and various sheet
		Failing load versus length for various pitch with 0.019 stiffener and various sheet
		Failing stress versus length for various pitch with 0.019 stiffener and various sheet.
		Failing stress versus length for various pitch with 0.029 stiffener and various sheet.
	აე.	Failing load versus length for various pitch with 0.009 stiffener and various sheet
		Individual Stiffeners
	<b>3</b> 6.	Failing stress versus $L/\rho$ for 0.029 stiffener with 0.019 sheet.
		EA SERIES CURVES
	37.	Failing stress versus stiffener depth for 0.009 stiffener with 0.009 sheet for various $L/\rho_{s}$
	<b>38</b> .	Failing stress versus stiffener depth for 0.009 stiffener with 0.029 sheet for various $L/\rho s_{}$
		Failing stress versus stiffener depth for 0.009 stiffener with 0.049 sheet for various $L/\rho s_{}$
		Failing stress versus stiffener depth for 0.029 stiffener with 0.009 sheet for various $L/\rho s_{}$
		Failing stress versus stiffener depth for 0.029 stiffener with 0.029 sheet for various $L/\rho s_{}$
		Failing stress versus stiffener depth for 0.029 stiffener with 0.049 sheet for various $L/\rho s_{}$
		Failing stress versus stiffener depth for 0.049 stiffener with 0.009 sheet for various $L/\rho_8$
		Failing stress versus stiffener depth for 0.049 stiffener with 0.029 sheet for various $L/\rho_8$
		Failing stress versus stiffener depth for 0.049 stiffener with 0.049 sheet for various $L/\rho_8$
		Failing stress versus stiffener width for 0.009 stiffener with 0.009 sheet for various $L/\rho_8$
		Failing stress versus stiffener width for 0.009 stiffener with 0.029 sheet for various $L/\rho s$
	10.	Failing stress versus stiffener width for 0.009 stiffener with 0.049 sheet for various $L/\rho s_{}$

			Page
FIGURE	49.	Failing stress versus stiffener width for 0.029 stiffener with 0.009 sheet for various $L/\rho s_{}$	<b>5</b> 3
	<b>5</b> 0.	Failing stress versus stiffener width for 0.029 stiffener with 0.029 sheet for various $L/\rho s_{}$	53
	51.	Failing stress versus stiffener width for 0.029 stiffener with 0.049 sheet for various $L/\rho s_{}$	54
	52.	Failing stress versus stiffener width for 0.049 stiffener with 0.009 sheet for various $L/\rho s$	54
		Failing stress versus stiffener width for 0.049 stiffener with 0.029 sheet for various $L/\rho s$	55
	54.	Failing stress versus stiffener width for 0.049 stiffener with 0.049 sheet for various $L/\rho s_{}$	55
		EC Series Curves	
	55.	Failing stress versus stiffener width and depth for 0.009 stiffener with 0.009 sheet for various $L/\rho_8$	56
	56	Failing stress versus stiffener width and depth for 0.009 stiffener with 0.029 sheet for various	-
	00.	L/ps	56
	57	Failing stress versus stiffener width and depth for 0.009 stiffener with 0.049 sheet for various	
	01.	$L/\rho_8$	57
	<b>5</b> 0	Failing stress versus stiffener width and depth for 0.029 stiffener with 0.009 sheet for various	
	90.		57
	50	Failing stress versus stiffener width and depth for 0.029 stiffener with 0.029 sheet for various	٠.
	<b>99.</b>	$L/\rho_8$	<b>5</b> 8
	co	Failing stress versus stiffener width and depth for 0.029 stiffener with 0.049 sheet for various	•
	ου.	Falling stress versus stillener width and depth for 0.029 stillener with 0.049 sheet for various $L/\rho_8$	<b>5</b> 8
	01	Failing stress versus stiffener width and depth for $0.049$ stiffener with $0.009$ sheet for various	•
	61.	Failing stress versus stiffener width and depth for 0.049 stiffener with 0.009 sheet for various $L/\rho_8$	59
		$L/\rho s$ Failing stress versus stiffener width and depth for 0.049 stiffener with 0.029 sheet for various	Ja
	62.	Faling stress versus stiffener width and depth for 0.049 stiffener with 0.029 sheet for various	59
	40	$L/\rho$ sFailing stress versus stiffener width and depth for 0.049 stiffener with 0.049 sheet for various	00
	63.	Failing stress versus stiffener width and depth for 0.049 stiffener with 0.049 sheet for various $L/\rho_8$ .	60
		L/ρ8	U
		I SERIES CURVES	
	64.	Failing stress versus $L/\rho$ for various $R/t's$	60
		Failing stress versus $R/t$ for various $L/\rho_8$	61
		Failing stress versus covering sheet thickness for 0.019 corrugation of 1-inch pitch and	
	0	various lengths	61
	67.	Failing stress versus covering sheet thickness for 0.020 corrguation of 2-inch pitch and	
		various lengths	61
	68.	Failing load versus covering sheet thickness for 0.020 corrugation of 1/2-inch depth having	
		1- and 2-inch pitch and various lengths	62
		D SERIES CURVES	
	69.	Failing stress versus $L/\rho$ for various stiffener thickness	62
•	70.	Neutral axis location and radius of gyration versus sheet thickness for 0.030 stiffener of A	
		section	63
	71.	Neutral axis location, area, moment of inertia, and radius of gyration versus stiffener depth	
		for 0.030 stiffener of EA section	63
	72.	Neutral axis location, area, moment of inertia, and radius of gyration versus stiffener width	
		for 0.030 stiffener of EB section	63
	73.	Neutral axis location, area, moment of inertia, and radius of gyration versus stiffener width	
		and depth for 0.030 stiffener of EC section	64

## AN INVESTIGATION OF THE COMPRESSIVE STRENGTH PROPERTIES OF STAINLESS STEEL SHEET-STRINGER COMBINATIONS

(Prepared by E. H. Schwartz and C. G. Brown, Matériel Division, Air Corps, Wright Field, Dayton, Ohio, Sept. 10, 1934)

#### SUMMARY

An examination of the test data indicates that-

- 1. The strength properties of sheet stringer combinations are governed principally by the strength properties of the stringer, and only to a minor degree by the sheet to which the stringer is attached, and that the highest structural efficiency from the point of view of carrying compressive loads is attained for a given stiffener by reducing the skin thickness to a minimum.
- 2. For a sheet-stringer combination the total load carried is a linear function of the stiffener units provided, and is independent of the stiffener pitch.
- 3. The effective width of sheet working with a stiffener at any stiffener stress, up to the stiffener failing stress for a given sheet thickness may be determined by the expression:

$$W = C \sqrt{\frac{E}{\sigma_*}} t,$$

the units being defined in the body of the report.

- 4. The coefficient C in the foregoing expression is a function, primarily, of sheet thickness. The data were insufficient to definitely indicate any dependence of C on stiffener failing stress, for a given sheet thickness.
- 5. Having once established a curve of stiffener  $\sigma_s$  versus  $L/\rho$ , the properties of any sheet-stiffener combination may be determined directly.
- 6. The properties of stainless steel columns follow a form of Euler-Johnson relationship, the position and shape of the curve for a particular section being a function of the cross section shape and thickness of that section and of the column fixity.
- 7. The properties of corrugated stainless steel sheets are governed by both slenderness ratio and R/t ratio, in such a manner that buckling values for a certain R/t ratio decrease with increasing  $L/\rho$ .
- 8. For a given stiffener shape and length its failing stress increases with thickness up to some limiting thickness after which it remains constant.
- 9. For a constant stiffener shape, thickness, and length, the load per stiffener in sheet-stiffener combinations increases parabolically with increase in sheet thickness.

- 10. For such data as were available the linear relationship between load carried and stiffener units, and independence from stiffener pitch applies as well to aluminum alloy plate stringer combinations.
- 11. The present specification for the strength of spot welds is not adequate to prevent failures of the type experienced during these tests. It is felt that it should be amended by a requirement to the effect that the weld strength of built-up sections be demonstrated by test to be sufficient to allow complete collapse, or crushing, of the section without weld failures.

#### **HISTORY**

The project was initiated in October 1931 and drawings and procurement data were completed by June 1932 at which time invitations for bids were issued.

The bids received at that time were so in excess of the funds available to carry out the project that the stiffener sections were changed from the original section which had no outstanding welding flanges at the back of the stiffener to the type of stiffener shown herein, in order to reduce the cost of the specimens to a point where procurement was possible with available funds.

New invitations were then issued and eventually a contract awarded to the Curtiss Aeroplane & Motor Co. for their fabrication.

The selection of stiffener cross section, gages, etc. was influenced by and based upon the data available at the time the project was initiated, namely 1931.

Certain sections presented herein, however, were decided upon at a later date to supply information of a specific nature, and are included herein to broaden somewhat the scope of the original project.

#### DATES AND PLACE OF TESTS

All tests were conducted between December 1933, and June 1934, as time and equipment were available. The majority of the tests were conducted at Wright Field, Dayton, Ohio, using the testing equipment of the Materials Branch. Tests on specimens requiring a load in excess of 100,000 lb./sq. in. to cause failure were tested at the Bureau of Standards, Washington, D. C.

#### **OBJECT**

The object of the project was to determine, insofar as possible, the effects of the many variables that enter into the determination of the compressive strength properties of the flat stainless steel sheet-stiffener combinations tested during the course of the investigation.

#### DESCRIPTION

#### Test specimens

The specimens were fabricated in accordance with Matériel Division drawings SK-18698, 18699, and 533064 from commercial 18-8 sheets, no Army specification on stainless steel being available at the time the speciments were made. The physical properties of the material, as determined by tests on specimens of representative thicknesses taken from the specimens, are tabulated in table I.

The sheet-stiffener combinations consisted of closed section stiffeners of the type shown in figures 1 and 2, electrically spot welded to sheets. The spot welding technique was determined by the practice of the contractor. The weld spacings were as follows:

Sheet thick-	Weld
ness	spacing
Inch	Inch
0.010-0.015	316
.020030	14
.050	36

The welding flanges were  $\%_6 \pm \%_4$  on thicknesses of 0.010 to 0.030, and  $\%_4 \pm \%_4$  on 0.050 sheets.

The allowable tolerance on the sheets were:

Sheet thick- ness	Tolerance
Inch	Inch
0. 010-0. 030	±. 015
. 031 051	±. 0025

The tolerance on the stiffener over-all dimensions was  $\pm \frac{1}{14}$  inch and on spacing was  $\pm \frac{1}{12}$  inch.

The allowable deviation from straightness was set at not to exceed  $\frac{1}{2}$  inch per foot of specimen length. The over-all length of the specimens was held to  $\pm \frac{1}{16}$  inch of that specified.

The ends of the specimens were ground flat and were liquid-cooled during grinding. A departure from parallel ends to the extent of 0.004 per inch width or depth of specimen or stiffener was allowed.

It proved practically impossible to secure perfectly flat ends on the specimens, and accordingly the sheet was allowed to depart 0.003 inch from flat, at a point midway between stiffeners, on sheets of 0.010 inch thickness, and 0.002 inch from flat at the midpoint on sheets of 0.050 inch thickness. Some of the first specimens received, that is, I series, were reground at the division to obtain flatter ends. The set-up used for regrinding is shown in figure 79.

A reasonable degree of flatness of the skin between stiffeners was required, and a satisfactory flatness was

furnished in practically all specimens. A twist of the finished specimens of not to exceed 0.01 inch per inch specimen length was allowed. This twist was usually readily removable at the time of testing.

The test series were divided into several parts, as follows:

Parts A and B.—The specimen length was held constant, and the stiffener thickness, sheet thickness, and stiffener pitch, and number of stiffeners varied in order. The stiffener cross section dimensions were held constant. Section shown in figures 1 and 78.

Part C.—In this series length effect was studied by controlling lengths for certain stiffener and sheet combinations. The stiffener cross section was held constant, being the same as the A series stiffener section. Five lengths each were arranged.

Part EA.—In this series the stiffener width was held constant, and the stiffener depth varied for several combinations of sheet and stiffener thicknesses, and for three lengths each. See figures 2 and 78 for typical stiffener section.

Part EB.—In this case the stiffener depth was held constant and the width varied, other factors being controlled as in part EA. See figures 2 and 78 for typical stiffener section.

Part EC.—In this series, both width and depth were allowed to vary, the other variables being as in part EA. Typical stiffener section shown in figures 2 and 78.

Part F.—This series was essentially as in EA, except that the method of attaching the stiffeners to the sheet involved four rows of welds on the sheet instead of two and that additional stabilizing grooves, spaced ½ inch, were provided in the stiffener sides. A section is shown in figure 2.

Part G.—A series of stainless steel cylinders of varying stiffener spacings and thicknesses for varying sheet thicknesses, lengths, and cylinder diameters. Stiffener section constant as in part A. (Not tested at date of writing.)

Part H.—A series of flanged angle section were used for stiffeners in varying depths, lengths, skin and stiffener thicknesses, and stiffener spacing. (Note.—Specimens were not tested, due to their inefficient nature.)

Part I.—Here the specimens consisted of corrugations of ½ inch depth, with the pitch of the corrugations varying for certain thickness and lengths. Various flat sheets welded to the corrugations to simulate wing covering were also investigated. Typical section shown in figure 3.

#### Description of apparatus

In order to eliminate any undesirable effects due to lateral shifting of testing machine tests, the jigs shown in figures nos. 74, 75, and 76 were used. The jig consisted essentially of four 8- by 8- by 2-inch steel plates, having alternate plates rigidly connected by accurately ground shafts sliding through bronze bearings in adjacent blocks in such a manner that a compressive load is applied between the inner blocks when a tensile load is applied to the outer blocks. Due to the close, sliding fit between the shafts and the blocks lateral movement of one end of a test specimen with respect to the other end was eliminated.

specimens were provided. A close-up view of a long specimen in the jig is shown in figure no. 76.

A 2-inch square bar, ground flat, was secured to the upper working block of the jig, and a 3-inch square bar, likewise ground flat, centered on a spherical seating block centered on the lower working block.

Strips of hardened aluminum alloy 2 inches in width, and 1/8 inch thick were placed between the ends of the specimens and the ground steel surfaces, in order to prevent damage and to assist in attaining a more uniform load distribution. Figure no. 77 shows typical impressions of specimens left on the strips after testing.

With the jig installed in a testing machine, the specimens were then loaded to failure, operating the machine as if for a tensile test.

A 40,000-pound capacity Amsler and a 100,000pound Olsen machine were used at Wright Field. 600,000-pound Olsen machine at the Bureau of Standards was used to load specimens exceeding the capacity of the other two machines.

It was noted during the tests that considerable energy was stored in the guide rods of the testing jig. When a specimen began to fail, the stored energy was often sufficient to cause the failure to proceed to completion in an explosive manner, which was somewhat undesirable for the study of the process of failure.

Areas were determined from the actual weight and length of the individual specimens, using as the unit weight of stainless steel 0.286 lb./cu. in., which was an average value determined by weighing several sheets of known dimensions.

#### DISCUSSION

#### A series

The variables that may be investigated in the A series of specimens are, for a constant stiffener shape and length, as follows:

- 1. Stiffener thickness (0.010 to 0.050) normal thickness.
  - 2. Sheet thickness (0.010 to 0.050) normal thickness.
  - 3. Stiffener spacing (1% to 15 inches).
  - 4. Number of stiffeners (2 to 9).

The actual thicknesses were close to 0.009, 0.014, 0.018, 0.019, 0.029, and 0.049. During the planning stages of the project it was expected that a useful variable might be found in percent reinforcement defined by the expression:

$$R = \frac{\text{Stiffener area}}{\text{Stiffener area} + \text{sheet area}}$$

where the stiffener area is that of one stiffener, and the sheet area is that included between the centers of the adjacent stiffeners.

The variable R, in the case of a specimen having but two stiffeners, obviously gives an erroneous indication of the existing conditions, and is of questionable value when applied to two stiffener test specimens. Accordingly, little faith has been placed on values indicated by many two-stiffener specimens.

Average failing stresses have been plotted against R in figures 18 to 22, inclusive. If the range of R is

Shafts of sufficient length to accommodate the longest | investigated, it will be noted that the average failing stress varies linearly with R, the position and slope of the resulting line being determined, for a given stiffener, by the skin thickness. The effect of an increase in stiffener thickness or sheet thickness for a given R is to increase the average failing stress.

In reference 8, it is concluded that the optimum conditions for proportioning a stiffened sheet is that in which the stiffener retains its alinement until the sheet is carrying the maximum load of which it is capable.

This conclusion is apt to be misleading, for it emphasizes the sheet as the more important element of the sheet-stringer combination. It unquestionably applies to the case of a weak stiffener attached to a thick sheet, which is an inefficient combination in comparison to the reverse case.

In cases of sheet-stiffener combinations of normal proportions, it is quite possible for stiffeners of closed section to remain intact and to continue to carry high stresses long after the sheet has reached and pressed its maximum load. Even in cases where a more nearly simultaneous failure of the elements occurs, the portion of the total load carried by the sheet is small in contrast to that carried by the stiffeners. As stiffener thickness increases, the average failing stress goes up, and the percentage of load carried by the sheet goes down. In general, it may be noted that as the load carried by the sheet increases, the average failing stress lowers.

Accordingly, it would appear that to obtain the highest average failing stresses, the stiffener should be the element that carries the greatest emphasis, and that the sheet should be governed by conditions other than its simple load-carrying ability.

It should be noted that the R curves of figures 18 to 22, inclusive, are for a particular stiffener section thickness and length and cannot be applied directly to any other section. Any variation in allowable stress as determined from an R curve must be due only to a change in stiffener spacing and not to a change in stiffener area. The reason being that the stiffener is by far the governing variable of the combination.

This may be readily noted by examination of figures 13 to 17, inclusive, and 8 to 12, inclusive, where in one case average failing stresses have been plotted against stiffener pitch for a constant stiffener and a varying sheet thickness, and next for a constant sheet and vary-of the sheet is noted. In the second case, however, an enormous variance in average stress due to a change in stiffener thickness is noted.

When plotting the failing loads of the specimens against the number of stiffeners, as in figures 5 to 7, inclusive, a remarkably consistent variation is noted in that every combination of sheet and stiffener the total load carried is directly proportional to the number of stiffeners, regardless of their spacing. This variation is consistent with the findings of reference 1 where it was noted that the strength of flat plates under edge compression was independent of their width.

This immediately suggests that each stiffener and an unknown width of sheet form a unit working at the failing stress of the stiffener. It also explains the dropping off of average failing stress with increased stiffener spacing indicated in figures 8 to 17, inclusive.

Due to the nature of the stiffeners, the assumption that the stiffener failing stress would remain constant with a change in sheet thickness appears to be justified. On the basis of that assumption, which will be discussed more fully later, an attempt will be made to determine the amount of sheet working with each stiffener at a particular stiffener failing stress.

Let

P=load, pounds, carried by the test specimen at failure, for N stiffeners.

 $\sigma_s$ =failing stress of stiffener and whatever width of sheet may be working with it at failure.  $A_{sh}$ =effective area of sheet per stiffener.

The following relationship may be established in terms of the test specimen dimensions:

$$P = [NA_{st} + (N-1)A_{sh} + 0.875t]\sigma_s$$

Solving for Ash

$$\mathbf{A}_{sh} \! = \! \frac{P}{(N \! - \! 1)\sigma_s} \! - \! \left(\frac{N}{N \! - \! 1}\right) A_{st} \! - \! \frac{0.875t}{(N \! - \! 1)}$$

The term 0.875t is included with the working area on the presumption, justified later, that the working width will be equal to or greater than the overall width of the stiffeners, and increases to 1.00t for 0.050 stiffeners.

In the above expression for  $A_{sh}$  the only unknown is  $\sigma_s$ . The definition of  $\sigma_s$  must be expanded to indicate a failing stress of a closed section where the closing sheet is of zero thickness, but still acts to supply the stabilizing forces necessary to prevent the stiffener from behaving as an open section.

In order to determine  $\sigma_a$  it was assumed that the effective width of sheet working with the stiffener was the overall width of the stiffener, % inch in the case of 0.029 stiffeners, and 1 inch in the case of 0.049 stiffeners. Assumed values of stress were then computed using a stiffener area including only the sheet between the stiffener overall dimensions, the number of stiffeners on a test specimen, and the specimen failing load.

These assumed stresses were then plotted against skin thickness and the resulting curves extrapolated to zero skin thickness to give a proper value of  $\sigma_*$  (curves not included).

In the calculations of  $\sigma_s$  an average stiffener area has been used, together with values of P taken from P versus N curves. The calculations of  $\sigma_s$  and  $A_{sh}$  are arranged in tabular form in tables II and III.

Figure 25 shows the variation of  $\sigma_*$  with stiffener thickness, and indicates a rapid increase of failing stress up to a thickness of about 0.030, where the curve abruptly flattens out. A similar behavior will be noted in figure 24 where average failing stresses for the B series stiffeners have been plotted against stiffener thickness for several sheet thicknesses.

In reference 4 will be found the expression—

$$W' = \frac{C}{2} \sqrt{\frac{E}{\sigma}} t$$

where

W' = one-half the width of sheet working with a stiffener

 $E = \hat{m}$  odulus of elasticity of material

 $\sigma_{\bullet}$ =stiffener stress t=sheet thickness

C=a constant

W=2W'

Having computed the effective width of sheet working with a stiffener at a stress  $\sigma_s$ , it requires but two steps to compute W/t and C as has been done in table III, using the expression—

$$C = \frac{W}{\sqrt{\frac{E}{\sigma_A}}}$$

The coefficient C has been plotted in figure 26 against sheet thickness for  $\sigma_s$  from 71,500 lb./sq.in. to 120,000 lb./sq.in. Various stiffener thicknesses and lengths were used to obtain the  $\sigma_s$  variation.

The curves 1 and 2 of figure 26 indicate, respectively, the lower and upper limits of C as indicated by the plotted points. Curve 3 represents an estimated mean value of C and is the value recommended for use.

It will be noted from the computation of C that thickness enters as  $t^2$ . Since it was not possible to maintain a constant t, as was noted under the description of the specimens, variations in t are felt responsible for a considerable part of the variation of figure 26. The greatest effect due to variation of  $t^2$  would be noted where the thickness is small, and it is in this range where the results are most scattered.

The results are also influenced by the proper determination of  $\sigma_{\bullet}$ . Since  $\sigma_{\bullet}$  was necessarily determined by extrapolation, it is subject to the errors of that process.

Considering the sources of error involved, it is somewhat surprising that the computed values of C are not even more scattered than the results indicate.

#### C series

The purpose of this series was to investigate the influence of length for nominal sheet and stringer thicknesses of 0.010, 0.020, and 0.030, with spacings corresponding to certain of those used in the A series of specimens.

In figures 29 to 32, inclusive, the specimen failing load has been plotted against length for several sheet and stiffener thicknesses, for lengths up to 18 inches. The shape of the resulting curves is influenced considerably by the stiffener, and somewhat by the sheet.

In figures 27 and 28 a value of load per stiffener, P/N, obtained by dividing the total load carried by the number of stiffeners on the specimen, has been plotted. For the 0.019 and 0.029 stiffeners the resulting curves indicate that the linear relationship between total load and number of stiffeners, established for the A series, applies for any length within the range of lengths tested.

Due to the erratic results of the tests on the 0.009 stiffeners, the linear relationship is not experimentally so evident. The shape of the curves of P/N versus length is not completely consistent in shape, the curves for the 0.029 stiffener being least so.

The curves of  $\sigma_s$  versus stiffener thickness show the same trend toward a constant  $\sigma_s$  after a certain t is reached that was noted from the A and B series tests.

In figures 33 and 34, where average failing stresses have been plotted against length, an Euler-Johnson relationship is indicated.

#### Fixity effects

In order to obtain some idea of the fixity coefficient C realized during the flat end tests, a series of individual 0.029 stiffeners was salvaged from 0.019 sheets after the A and C series tests had been completed. The sheet was sheared off at the edges of the stiffener welding

The individual stiffeners were then tested as both knife-edge and flat-end columns. The results are presented in table IV and figure 36. On the latter figure Euler curves for C=1 and C=3, based on E=26,000,000, which is a fair average value, were plotted.

For the knife-edge tests the data obtained show a satisfactory tangent to the Euler curve at  $L/\rho = 90$ . Insufficient test data were available, however, to follow the flat end curve down to a well established junction with the Euler curve. The curve drawn, therefore, is not final, but it does not appear to be unreasonable and indicates that the assumption of C=3 for flat end tests is not greatly in error.

The use of previously tested stiffeners may be open to question. Every precaution, however, was used in selecting only undamaged stiffeners.

The curves indicate an Euler-Johnson or similar relationship between failing stress and slenderness ratio. The position of the curve for any other stiffener thickness below 0.029 would change, being lowered, and considerably flattened, judging by the tests on other stiffener sections.

In applying the results of tests on flat ends to a definite structure such as a box section wing, it does not appear unreasonable to apply a correction to flat end test results to bring them to whatever value of fixity is assumed, or determined to exist in the actual structure. If figure 36 were a design column curve, for instance, and it was determined experimentally that a coefficient of 2 was all that could be allowed, a curve midway between those drawn would become the design curve for C=2. The procedure would be similar for C=1, etc. In this respect, published data justifying the use of C=2 or more are few for the usual box wing construction where the sheer webs are widely separated. Values of 1.5 or below are more representative. The coefficient increases somewhat when multiple webs exist, reaching values sometimes in excess of 2. Lacking experimental verification, values in excess of 1.5 appear unconservative. The application of flat end tests directly without a proper fixity allowance is also unconservative.

Since the ratio of stress for C=3 to the stress for C=1 varies from one at  $L/\rho=0$  to 3 in the Euler range. the seriousness of flat-end allowables is less in the short column range where the majority of compression members are likely to fall.

#### Effective radius of gyration

The establishment of the sheet-stiffener units, or the linear relationship between total load and number of ener tests suggests an investigation to determine the possibility of a relationship between the two.

Accordingly, from table III a 0.029 stiffener series of data was selected for examination, as the values of Ccomputed for that data most nearly agreed with the curve of C finally drawn on figure 26. From the data, the width of sheet working with the stiffener was determined and the radius of gyration of the unit of sheet and stiffener was computed, using the moment of inertia of the unit about an axis through the unit center of gravity and parallel to the sheet.

The computations are tabulated in table VI. It will be noted that although the width of sheet working with the stiffener may vary as the sheet thickness varies, the radius of gyration of the unit remains nearly constant. The computed values of slenderness ratio ranged from 41.5 and 46 for a 91/4-inch test specimen for which the failing stress  $\sigma_a$  had previously been established, as compared to 46 for a 91/4-inch open section stiffener.

Referring to figure 36 it will be noted that for flatend tests, a stress of 115,000 lb./sq.in. is indicated at a slenderness ratio of 43, which value is neatly bracketed by the foregoing values. Thus it is indicated that the column properties of the sheet stiffener units may be determined from a column curve for the individual stiffeners.

The above comparison may be a bit confusing if it is recalled that  $\sigma_a$  is based on a stiffener closed by a sheet of zero thickness whereas the plotted curves are for a 0.019 closing sheet. The stress  $\sigma_{\bullet}$ , however, is probably very nearly constant for varying closing strips and was previously assumed so. The curves used in the establishment of  $\sigma_{ij}$  too, were usually nearly flat below 0.020inch thickness of closing strip. While curves of P versus L or P/A versus L would show a separation due to closing sheet thickness for the individual stiffener tests, a curve of P/A versus  $L/\rho$  would probably show little or no influence of closing strip, except possibly for short lengths and the curve for 0.029 stiffeners with 0.019 closing strip may be considered a representative column above.

The nature of the  $\sigma_*$  curves of figure 25 and the expression for effective width of sheet indicate that the effective width coefficient C should be applicable to other lengths than the 914-inch length of the specimens that determined it.

#### Constant o.

The assumption of a constant stiffener failing stress for a given stiffener in the presence of varying sheet may be challenged on the basis of the different types of failure that occurred as the sheet thickness varied.

If anything, the assumption is conservative for practical thicknesses. It will be noted from the curves of total load versus N, that for a given N the load carried varies approximately parabolically with skin thickness. If the stiffener failing stress is constant the increase in load carried must be due to an increase in the area of sheet working with the stiffener. Conversely, if the area of sheet working with the stiffener is constant, the failing stress of the stiffener must increase enormously to account for the rapid rise of load with skin stiffeners, and the column curves from the single stiff- | thickness. Due to the nature of the stiffeners, and the

susceptibility to local or elastic failures, it is not felt that the latter course is likely to be followed, and that the former is the more likely. It will be granted that there is perhaps a path some where between the extremes and that the stiffener failing stress may increase slightly with increased skin thickness up to the point where failures are governed by the outstanding welding flanges. Since the intermediate path cannot be ascertained, it is felt desirable to proceed with the assumpttion of constant  $\sigma_a$ .

It is likely that in the case of stiffeners rolled or drawn from a single strip, and having no outstanding welding flanges, the greatest variation of  $\sigma_*$  with sheet thickness would occur.

The parabolic relationship between load and skin thickness for a constant stiffener thickness suggests that extrapolating the parabola down to zero skin thickness would be a simpler procedure for determining  $\sigma_a$  in the presence of a sheet of zero thickness than the procedure used previously.

#### Material

Failing stresses were undoubtedly influenced to some extent by variations in the physical properties of the material, inasmuch as from table I it will be noted that moduli of elasticity varied considerably as did the proportional limit of the material. The extent to which these variables influenced the results is not felt to be large, but it is at the same time indeterminate, as it was not practicable to determine the characteristics of each and every specimen. There could be nearly as great a variation in the properties of the individual elements of the specimens as there is indicated for the material in general, as the stiffeners and sheet could not come from the same stock.

#### Application to other stiffeners

The results thus far presented have been based on flat specimens employing closed section stiffeners of a particular type.

Neither specimens nor data are available to determine the validity of the application of the results to open section stiffeners attached to the sheet by a single line of welds instead of the two lines used in the test specimens. The effect of curvature was not investigated due to lack of time and facilities for the investigation of the cylindrical specimens.

It appears quite likely that the effective width would decrease somewhat for stiffeners attached by a single line of welds.

Comparison with aluminum alloy sheet-stringer tests

As a matter of curiosity the results of an extensive series of tests on flat ended aluminum alloy plate stringer combinations were examined to ascertain whether or not the linear relationship between load and number of stiffeners applied to the aluminum-alloy specimens. Due to the confidential nature of the data, it cannot be presented herein. Only a few of the results permitted plotting of load against number of stiffeners. The results, however, were such as to verify the linear relationship rather conclusively for the particular stiffener section, sheet thickness, length, and rivet

pitch used. All of the latter variables will undoubtedly effect the total load carried, or the average failing stress, but for particular combinations, it appears that the behavior determined for the stainless-steel specimens applies also to the aluminum-alloy specimens.

The same data afforded a rough comparison of the merits of 24ST aluminum alloy and stainless steel plate stringer combinations: The comparison was made on the basis of the average stress developed in sections of the same pitch, same slenderness ratio of the stiffeners alone, and equal thickness of sheet and stringer, 0.018 for the steel and 0.050 for the aluminum alloy, the ratio of the latter values being closest to the ratios of unit weights of the materials.

Reduced to an  $L/\rho$  of 46, corresponding to the 9½-inch A series specimens, and a 6-inch stiffener spacing, it could be expected that average stresses of 67,500 lb./sq. in. and 26,870 lb./sq. in. could be developed for the two materials.

The ratio of these two is 2.51 which falls short of the 2.86 necessary for equal efficiency by about 14 percent, the aluminum alloy making the better showing. The above comparison, however, is not absolute, as the stiffeners were of radically different cross section. The aluminum alloy stiffener sections were of one piece with no outstanding flanges except at the attachment to the sheet. The rivet pitch was ¾ inch. Should the latter figure be increased, a more favorable comparison would be likely to result. Likewise, if the stainless-steel stiffeners were of a cross section similar to the aluminum alloy in freedom from outstanding flanges, a more favorable comparison would be likely.

In general, the stainless-steel combinations do not appear to have any hopeless or unsurmountable disadvantages as compared to aluminum alloy on the basis of structural strength.

#### Application to box sections

A question will doubtless arise as to whether the design of a box beam for instance should be based on an average stress basis, or on the basis of the  $\sigma_{\bullet}$  stress developed on the most stressed stiffener, and the effective width of sheet working with it.

In order to investigate this point in a simple manner the section shown in figure 4 will be analyzed on the two bases.

Computing the apparent moment of inertia of the section, assuming all sheet effective in compression:

$$\frac{I_{oo}}{2} = 0.010 \times 25.87 \times 7.5^2 + 6 \times 0.0855 \times (7.5 - 0.333)^2$$

$$I_{oo} = 82 \text{ in.}^4$$

The failing stress of the 0.030 stiffener will be 115,000 lb./sq. in.  $=\sigma_s$ .

A mean value of E of 26,000,000 lb./sq. in. will be used.

$$W = C\sqrt{\frac{E}{\sigma_A}} t$$

for t=0.010 C=10, from figure 26

$$W = 10\sqrt{\frac{26 \times 10^6}{1.15 \times 10^5}} \times 0.010$$

=1.505 in

$$I_{oo} = (0.010 \times 5 \times 1.505 + 0.875)7.5^2 + 26.4 + 41$$
  
= 4.74 + 26.4 + 41 = 72.14 in.4

Computing the new C. G. location

$$d = \frac{+0.2587 \times 7.5 - 0.010 \times 8.40 \times 7.5}{0.2587 + 12 \times 0.0855 + 0.0840}$$
$$= \frac{7.5 \ (0.2587 - 0.0840)}{1.3707} = 0.955 \ \text{in}.$$

The effective moment of inertia about an axis through the new C. G. is

$$I_{xx} = I_{oo} - Ah^2$$
  
= 72.14 - 1.3707 (0.955)<sup>2</sup>  
= 70.89 in.<sup>4</sup>

On the basis of average P/A the 0.030-0.010-5-inch pitch combination could be expected to develop for C=3 conditions 86,500 lb./sq. in.

The 0.030 stiffener on the individual stringer basis could be expected to carry 115,000 lb./sq. in.

Let us now investigate the moments that could be developed for the two cases:

Section modulus

$$\frac{Y_1}{I_1} = \frac{7.5}{82} = 0.0915 \qquad \frac{Y_2}{I_2} = \frac{8.455}{70.89} = 0.119$$

$$M = \frac{S}{Y} = \frac{86,500}{0.0915} \qquad = \frac{115,000}{0.119}$$

$$M_1 = 945,000 \qquad M_2 = 966,000$$

to cause failing stress.

From the foregoing simple example the difference in moments developed for the trial section on the two bases are so small (2 percent) that it may be concluded that either method would lead to the same result.

The effective EI of the beam, however, would be most nearly represented by the lower of the two EI's for purposes of calculating deflections.

The assumption that the modulus of rupture at failure, in bending, could be computed from the expression  $\frac{My}{I}$ , which is an almost universal procedure, is verified by test. To obtain consistency with compression machine results on small specimens, however, due account must be taken of length and end fixity.

#### Variation of stiffener cross section

The EA, EB, and EC series tests were for the purpose of investigating effects, due to varying the stiffener dimensions.

It was originally intended that as the stiffener dimensions varied the length would also be varied to maintain a constant slenderness ratio based on the properties of the stiffener alone, that being the only possible variable that was contemplated at the time. The slenderness ratios desired were 25, 50, and 75. In the process of adding welding flanges to the sections finally used, exact control of the length was lost with the result that the slenderness ratios as computed for the final sections and the lengths furnished were of the order of 23.2–24.8, 46.4–49.6, and 69.5–74.4 for the series.

#### EA series

This series of tests was intended to supplement the A series tests by determining the influence of variations of the stiffener dimensions. The specimens were originally chosen to have a constant slenderness ratio, based on stiffeners alone. Accordingly, as the stiffener depth increased, the specimen length increased.

It has been shown in the preceding discussion that the governing slenderness ratio is not that involving the  $\rho$  of the stiffener alone, but is that determined by the sheet that is working with a stiffener. Accordingly, the original selection of constant stiffener  $L/\rho$  was not entirely satisfactory for the purpose intended.

For the 0.009 stiffener EA specimens a decrease in average failing stress is noted as the stiffener depth increases. This may logically be charged to decreasing stability of the stiffeners as the depth increases. The influence of slenderness ratio and sheet thickness are not pronounced for these thicknesses.

The 0.029 stiffener EA specimens show a much higher average failing stress than the 0.009 specimens, an appreciable influence due to slenderness ratio, but little influence due to variation of the sheet thickness. The behavior of the 0.049 stiffener EA specimens was similar to that of the 0.029 with a generally slightly higher average stress developed. In these tests, again it was evident that the stiffener is primarily responsible for the strength properties of the specimens.

The 0.029 stiffener EA specimens indicate a tendency toward a maximum average stress for a stiffener of %-inch depth, whereas the 0.049 stiffeners indicate a possibility of a maximum average stress for stiffeners of about %- to 1-inch depth for a %-inch width.

The maximum stresses appear, for a given stiffener, to decrease somewhat as the sheet thickness increases. With all thicknesses of stiffeners, it was noted during tests of the EA specimens that at depths of 1 inch or more the predominating type of stiffener failure was by lateral buckling of the stiffener as a whole rather than a collapse of its component parts.

#### EB series

The 0.009 stiffener EB specimens show little variation of average failing stress for any of the variables entering. For the 0.029 stiffener specimens there is apparent a decrease in average failing stress with increasing stiffener widths, the decrease, however, is greatest for the 0.009 skin and practically disappears for the 0.049 skin. Length appears to have an appreciable effect.

For the 0.049 stiffeners the same behavior was noted as for the 0.029 stiffeners, with the length effect increasingly apparent.

#### EC series

It would appear from an examination of the EC series of data that the effect of increasing both the stiffener width and depth might be expressed as the summation of the effects of increasing first the depth and secondly the width.

The data indicate that the slope of the curves of the EC data for 0.009 stiffeners are approximately the sums of the slopes of corresponding curves of the EA

and EB data. For the higher stiffener thicknesses the average failing stresses correspond closely with those of the EA series, and the influences of the independent variables appear the same.

#### Open section stiffener tests

The results of flat and column tests on stiffeners typical of the A, B, and C series stiffeners are shown in figure 69.

The open section tests indicate an appreciable increase in failing stress with increase in thickness and do not exhibit the tendency of constant failing P/A for thicknesses in excess of 0.029 determined from the closed section specimen tests.

The failures for the longer lengths were predominantly due to twisting. The results do not appear at all useful in predicting the strength properties of platestringer combinations and are presented only as data on particular open sections.

#### Formation of wrinkles

It was noted during the tests that on the lighter skins the formation of wrinkles started almost at no load, and was influenced by the width or spacing of weld lines. The wrinkle pattern was generally such that pitch of the wrinkles was equal to the distance between weld lines. In other words, the wrinkle pattern formed a series of squares.

Wrinkling loads were practically always lower by considerable amount than failing loads. In the case of some of the heavier gages of the EA, EB, and EC series stiffeners, another type of wave was observed, involving the reinforcing groove at the side or back of the stiffener. In this case, the formation of waves only slightly preceded failure, and was largely responsible for it. In this case, the pitch of the waves was 3 to 4 times the width of the stiffener side. It was this type of waving that usually precipitated explosionlike failures in which the sections burst as if blown apart over a considerable length. The waves undoubtedly imposed very severe loads on the welds, and the welds were likely unable to hold the elements in place at the advanced stages of waving. See figure no. 76.

Dimensions of outstanding legs

Nominal t	b	b/2t
0. 010 . 015 . 020 . 030 . 050	0. 187 . 187 . 187 . 187 . 187 . 250	6. 35 4. 23 3. 18 2. 12 2. 50

Dimensions of flat across A section stiffeners

Nominal t	W	0.5/t
0. 010 . 015 . 020 . 030 . 050	0. 5 . 5 . 5 . 5	50 33.3 25 16.7

#### Types of failure

The type of failure showed a general classification into several fairly well defined groups. These are listed and described briefly as follows:

Type A.—A failure due to the buckling of the outstanding legs of the stiffener. It was confined usually to the thinner stiffeners. The appearance of the failures is well illustrated at the top and bottom of the right-hand specimen of figure no. 83, and by figure no. 80.

Type B.—When the combined thicknesses of the stiffener and sheet were less than twice the stiffener thickness, the failures were generally precipitated by buckling of the stiffener welding flanges. This type of failure was common to the specimens having very thin sheet. The appearance of this type of failure is shown in figure no. 81.

Type C.—When the stiffener dimensions became sufficient, the sides or backs of the stiffeners became unstable, permitting the formation of waves in those elements which led to their collapse. This type of failure is shown in figures nos. 82 and 90.

Type D.—When a heavy sheet was used in the presence of light stiffeners, a failure such as shown in figure no. 83 occurred. In this figure it may be noted how the heavy sheet has bowed toward the stiffeners, and how the outstanding legs of the stiffener at the failed section have bowed simultaneously toward the sheet.

Type E.—When extreme differences between sheet and stiffener thickness existed, failure often occurred due to the pulling of slugs of welded material from the lighter sheet by the heavier, as may be seen in figure no. 84. This failure is not due to faulty welding but due to too few, or too small welds, and is a somewhat abnormal condition.

Type F.—Due to the formation of severe buckles in the sheet, failure of stiffeners sometimes occurred due to the distortion of the stiffener welding flanges by the sheet buckling. The appearance of the failed specimens was practically the same as that shown in figure no. 85.

Type G.—This failure was due to column failure of the specimens, and was noted before complete collapse usually in the heavier stiffener sections. This type of failure is shown in figures nos. 86 and 90.

Type H.—When there was a rough balance of the elastic stability of all of the elements of the specimens the resulting failure was a general collapse of the complete section. This is illustrated by figure no. 87.

Type I.—When, as in the EA series, the ratio of depth to width of the stiffener exceeded about 1.5 the failure was due to lateral instability of the stiffeners, and was characterized by a waving of the stiffener in a direction parallel to the sheet. Figure no. 88 shows a typical failure of this type.

Type J.—This classification includes all failures due to poor welds and weld failures, due usually to the lack of fusion of the welded material. The greatest portion of these failures occurred when thicknesses of 0.030 and 0.050 were encountered. The poor spot welds and complete disintegration, due to poor welds, will be noted in figure no. 89. Figure no, 90 shows a side view of the failure.

fied, and typical buckling failures are shown in figures no. 91 (corrugated sheet alone) and no. 93 (corrugated sheet with flat sheet attached).

Typical column failure for the corrugated sheet is shown in figure no. 92 and for the corrugated sheet with flat sheet attached in figure no. 94.

#### Weld failures

In the EA, EB, and EC series, particularly for the thicker sheets, considerable difficulty due to poor welding was encountered. Due to the sudden nature of the failures in the majority of cases, it was not always possible to detect weld failures prior to a general exploding of the specimen.

For this reason, it is believed that the scattering of many of the test points is due to weld failures that were not detected. In cases where welds failed prior to a general failure of the specimen, the test was discontinued and a gap in the data exists.

It was rare that a failed weld showed evidence of pulling a slug of welded material from the thinner of two sheets joined together. The majority of the failed welds showed a very small fused area in comparison to the electrode diameter. External appearances of the welds gave no indication of the condition of the welds. In some instances it was noted after failure that attempts had been made to weld through a layer of paint applied along a line of welds due to a specimen number painted on a sheet. Also it was noted on a number of specimens that the specimens had been rewelded over the original welds.

The spot-welding ability or technique of the contractor is held responsible for the failures; and spot welding of stainless steel, as a whole, should not be condemned due to the unsatisfactory nature of the welds experienced during these tests.

Judging from tests on specimens procured from the same and other sources, it is quite possible to so weld the specimens used for these tests that no welds would have failed in sheets of similar thicknesses no matter what the degree of crushing of the specimens might have been.

Since the initiation of the project, Air Corps Specification No. 20011, dated January 27, 1934, has been issued. The essential items of the specification are the requirements of thyratron-tube control and the establishment of standard weld strengths for specific sheet thicknesses to be determined by tension tests on strips joined by a single weld.

The use of a thyratron control is excellent but not essential, and it might be mentioned in passing that the contractor used a thyratron control for timing the welds, and during the later stages of fabrication checked welds for consistency by tension tests and still the welds failed under compressive loads.

Accordingly, it is believed that specification no. 20011 is inadequate in itself in guaranteeing freedom from weld failures on members subject to compression and that it should be amended, or procurement specifications or contracts so worded as to require that in addition to compliance to specification no. 20011 a contractor must demonstrate by suitable compression

The failures of the corrugated sheets were not classi- | tests that welded, built-up sections of his design are capable of withstanding crushing after reaching their failing load without weld failures.

#### I series corrugations

It was hoped that a single curve for the ultimate compressive or buckling stress for the corrugated sections could be established in terms of R/t, R being the radius of curvature of the corrugations and t the thickness. It was found that R/t and  $L/\rho$ , or slenderness ratio, were interdependent and that column curves could be established for certain R/t's and buckling curves for certain  $L/\rho$  ratios, but that no one curve would express an upper limit.

It will be noted that the curves for certain R/t values have been drawn tangent to a Euler curve for C=3, in figure 64.

Previous tests on aluminum-alloy corrugations had indicated that flat-end test conditions closely approximated the end conditions for a fixity coefficient of 3.

In table VII a comparison is made between the strength properties of stainless steel corrugated sheet 24ST aluminum alloy corrugated sheet. The comparison is made on the basis of equal weights of cross section

for identical slenderness ratios of 35, 70, and 100.

The ratio of weights is  $\frac{0.286}{101}$  or 2.83. In selecting stresses for stainless steel, the R/t has been increased by multiplying by 2.83, corresponding to an equivalent reduction in thickness.

For sheets of aluminum alloy or stainless steel to carry equal loads, the stress developed in the stainless steel must be 2.83 times that developed in the aluminum alloy. Hence, in table VII whenever the ratio of stresses exceeds 2.83 steel offers an advantage in strength over 24ST aluminum alloy sheet of equal weight.

In selecting sections on the basis of equal strength, a more proper basis, the weight ratio would not be quite as large as the strength ratio on the basis of equal weight due to the effect of reducing the thickness of the steel somewhat to bring the strength ratio to one.

In the foregoing table no account has been taken of the presence of any covering skin, inasmuch as for the purposes of this discussion, covering skins of proportional thicknesses are in order. One item that will operate against aluminum alloys in comparing with stainless steel is that of protective coatings, which may reach as much as 10 percent of the aluminum alloy weight for thin sheets.

In order to investigate the effect of covering skin on the average stress developed in covered corrugations, flat skin of varying thickness was welded to the corrugations and the resulting specimens loaded in compression.

It was noted throughout these tests that the flat covering skin became wrinkled at very low loads, and that the wrinkle pitch was, on an average, close to the pitch of the corrugations, regardless of the cover skin thickness.

It will be noted from figure 66 that the average failing stress drops off at a rate of about 20,000 lb./sq. in. per 0.01 inch of covering thickness up to a thickness of

about 0.020 inch for 1-inch pitch corrugations, after which the average stress begins to rise again. It is unfortunate that the 0.029-inch thickness was not great enough to determine how far this rise might be carried.

In figure 68 failing loads have been plotted against covering thickness. It will be noted that the total loads vary in a peculiar manner, increasing as the covering thickness increases, in spite of a lowering of the average failing stress.

The data are not as consistent as might be desired, due to the fact that this series of tests was performed by inexperienced personnel, and were the very first tests conducted during the investigation.

#### Suggestions for future work

Insufficient time has been available to fully investigate all the points suggested during the examination of the available data. The following topics are suggested to anyone interested in a continuance of the investigations, or who may be engaged in a similar investigation:

- 1. The influence of stiffener sections of other shapes, notably sections having as outstanding legs only the flanges necessary to weld it to the sheet.
- 2. The development of a most generally efficient stiffener section.
- 3. An investigation of the influence of stiffeners attached by a single row of welds on the effective width of sheet acting with the stiffener.
- 4. An investigation of the possibility of determining effective width of sheet in the case of riveted sheet stiffener combinations, particularly in aluminum alloy.
- 5. An investigation to determine the effect of curvature on the effective width of sheet working with the stiffener.

#### NOTE

Since the writing of this report, completed about June 1, 1934, those specimens that failed, due to weld failures, have been replaced by the vendor.

The results of tests on these replacement specimens will be presented in an appendix to this report as soon as the data become available, together with data on sheet-stringer combinations involving stiffeners of sections differing from those used in the main investigation.

#### REFERENCES

- 1. N. A. C. A. Technical Report No. 356, Strength of Rectangular Flat Plates Under Edge Compression by Schuman and Black.
- 2. N. A. C. A. Technical Report No. 473, Strength of Thin Walled Cylinders in Compression by Lundquist.
- 3. Applied Elasticity, Prescott.
- 4. A. C. T. R., Serial No. 3739, An Investigation of the Available Information on the Strength Properties of Reinforced Skin Construction by C. G. Brown.
- 5. C. I. T. Paper, The Ultimate Strength of Thin Flat Plates in Compression by E. E. Sechler.
- 6. N. A. C. A. Technical Note No. 445, Comparison of Three Methods for Calculating the Compressive Strength of Flat and Slightly Curved Sheet and Stiffener Combinations by E. E. Lundquist.
- 7. Matériel Division, E. S. M. R., Serial No. Str-51-26 (Addendum 2), Compressive Tests on Miscellaneous Stainless Steel Sections for P-26 Stainless Steel Wings by S. R. Carpenter and C. G. Brown.
- 8. M. I. T. Paper, Notes on the Design of Metal Parts for Use in Aircraft Construction.

Table I.—Physical properties of test specimen material
[Material Branch tests, reference series R34-93]

Specifi-			Proport	ion limit	Yield point	Elongation	Modulus
cation	Thickness	U. T. S.	Tangent	0.0001 inch per inch	(0.002 inch per inch)	(percent in 2 inches)	elasticity (1,000 lb./ sq. in.)
1 1 2 2 3 4 5 6	0.0090 .0090 .0090 .0094 .0148 .0147 .0144 .020 .020 .0290 .0307 .0288 .0494 .0473 .0490	176, 980 191, 840 183, 300 188, 800 187, 700 188, 800 191, 400 191, 400 184, 500 185, 200 175, 800 171, 700 180, 160 180, 160	55, 440 60, 990 54, 240 53, 080 57, 350 54, 350 84, 390 83, 200 55, 550 48, 480 50, 630 48, 880	72, 100 94, 300 67, 700 87, 500 77, 550 64, 550 92, 800 107, 490 76, 500 64, 700 69, 650 77, 400	163, 000 157, 460 168, 150 144, 400 157, 300 154, 210 165, 400 164, 300 164, 300 164, 300 161, 700	3.55 6.50 6.55 6.55 5.55 5.50 12.00 5.55 7.60	26, 720 27, 780 27, 780 29, 850 29, 020 31, 030 27, 430 28, 700 25, 220 24, 840 28, 450

Table II.—Computation of stiffener failing stress
LENGTH, 9.25 INCHES

#### 0.009 STIFFENER

Sheet thickness (inch)	Sheet area (inch)	Stiffener area (inch)	Total area (1 stiffener)	Failing load (5 stiffeners)	Stiffener failing stress (lb./sq. in.)	σ,
0.009 .014 .018 .028 .048	0.0079 .0122 .0158 .0245 .0420	0. 0286 . 0286 . 0286 . 0286 . 0286	0. 0365 . 0408 . 0444 . 0531 . 0706	14, 000 16, 000 20, 000 27, 700 38, 000	76, 700 78, 300 90, 200 104, 300 107, 600	75, 000
		0.0	14 STIFFEN	ER		<u>'</u>
0.000	0.0079	0.0424	0.0503	24 300	98.600	h

	0.009 .014 .018 .029 .048	0. 0079 . 0122 . 0158 . 0254 . 0420	0. 0424 . 0424 . 0424 . 0424 . 0424	0. 0503 . 0546 . 0582 . 0678 . 0844	24, 300 28, 000 31, 400 41, 000 54, 000	96, 600 102, 200 108, 000 121, 000 128, 000	90, 000	
--	---------------------------------------	---	---	---	---	---	---------	--

#### 0.019 STIFFENER

0.009         0.0079         0.0548         0.0627         35,000         111,800           .014         .0122         .0548         .0670         39,300         117,200           .018         .0153         .0548         .0706         42,200         120,000           .029         .0254         .0548         .0802         54,000         134,000           .048         .0420         .0548         .0968         73,500         152,000	115, 000
---	----------

#### 0.029 STIFFENER

400 000 000	52, 000 117, 000 54, 200 116, 400 58, 300 120, 000 70, 000 131, 000 95, 000 136, 000	0. 0888 . 0931 . 0971 . 1063 . 1329	0. 0809 . 0809 . 0809 . 0809 . 0809	0.0079 .0122 .0162 .0254 .0420	0.009 .014 .018 .029 .048
-------------------	--	---	---	--	---------------------------------------

#### 0.049 STIFFENER

0.009 0.009	0. 144 0. 153	88, 000	115, 000	115,000
.014 .014	. 144 . 158	91, 000	115, 000	
.018 .018	. 144 . 162	95, 000	117, 200	
.029 .029	. 144 . 173	112, 500	130, 000	
.048 .048	. 144 . 192	145, 500	151, 500	

#### LENGTH, 3 INCHES

			, , ,		
Sheet thickness (inch)	Stiffener thickness (inch)	Total area (1 stiff- ener)	Failing load per stiffener	Stiffener failing stress (lb./sq.in.)	σ,
0.009 .019 .029	0.019	0.0627 .0716 .0802	8, 030 9, 000 11, 200	127, 800 125, 600 139, 500	125,000
.009 .019 .029	. 029	.0888 .0977 .1063	13, 140 14, 380 16, 200	148, 100 147, 000 152, 400	145, 000
., , <del>.</del>	I	LENGTH,	6 INCHES	<u> </u>	
0.009 .019 .029	0. 019	0.0627 .0716 .0802	7, 750 8, 700 10, 930	123, 400 121, 300 136, 000	120,000
. 009 . 019 . 029	. 029	.0888 .0977 .1053	11, 800 13, 080 15, 460	133,000 134,000 145,100	131,000
		LENGTH,	12 INCHE	S	
0.009 .019 .029	0.019	{ 0.0627	6, 450 7, 620 9, 400	102, 800 106, 400 117, 200	102, 500
. 009 . 019 . 029	.029	.0888 .0977 .1063	9, 170 10, 470 12, 910	103, 200 107, 200 121, 500	103, 000
		LENGTH,	18 INCHE	S	
0.009 .019 .029	0.019	0.0627 .0716 .0802	4, 650 5, 750 7, 000	74, 100 80, 300 87, 200	71, 500
. 009 . 019 . 029	.029	.0888 .0977 .1063	6, 530 7, 900 8, 550	73, 500 75, 700 80, 200	73, 200

 ${\bf TABLE\ III.} - Computation\ of\ effective\ width\ of\ sheet\ working\ with\ stiffeners$ 

A SERIES TESTS N=5 Stiffener t=0.009  $\sigma_{*}=73,000$   $\sqrt{\frac{E}{\sigma_{*}}}=18.88$ 

t	P	$\frac{P}{(N-1)\sigma_{\bullet}}$	$\frac{N}{N-1}A_{ii}$	$\begin{array}{c} 0.875t \\ \overline{N-1} \end{array}$	Ash	w	$\frac{W}{t}$	С
0.009 .014 .018 .028 .048	14,000 16,000 19,000 28,000 38,000	0. 0479 . 0548 . 0651 . 0959 . 1300	0.0358	0.0020 .0031 .0040 .0063 .0108	0. 0101 . 0159 . 0253 . 0538 . 0834	1. 121 1. 137 1. 405 1. 920 1. 735	124. 5 81 78 68. 5 36. 1	6. 6 4. 3 4. 1 3. 6 1. 9
		St	tiffener $t = 0.01$	4 σ <sub>e</sub> =90,000	$\sqrt{\frac{E}{\sigma_i}} = 17.0$	0		
0.009 .014 .018 .029 .048	24, 500 28, 000 31, 400 41, 000 54, 000	0, 0675 . 0778 . 0872 . 1138 . 1500	0.0530	0.0020 .0031 .0040 .0063 .0108	0. 0225 . 0217 . 0302 . 0545 . 0862	2. 50 1. 55 1. 68 1. 88 1. 79	278 111 93 65 37. 5	16. 4 6. 5 5. 4 3. 8 2. 2
		St	iffener $t=0.01$	$\sigma_s = 105,00$	$\sqrt{\frac{E}{\sigma_e}} = 15.$	75	·	
0.009 .014 .018 .029 .048	35, 000 39, 300 42, 200 54, 000 73, 500	0. 0834 . 0937 . 1005 . 1285 . 1750	0.0675	0.0020 .0031 .0040 .0063 .0108	0. 0139 . 0231 . 0290 . 0547 . 1067	1. 55 1. 65 1. 61 1. 88 2. 27	171 118 90 65 47.3	10. 8 7. 5 5. 7 4. 1 3. 0
		St	iffener $t=0.04$	9 $\sigma_s = 115,00$	$\sqrt{\frac{E}{\sigma_{\bullet}}} = 15.$	02		
0.009 .014 .018 .029 .048	87, 500 92, 500 96, 000 110, 500 145, 500	0. 1900 . 2010 . 2085 . 2400 . 3180	0. 180	0. 0022 . 0035 . 0045 . 0073 . 0120	0. 0078 . 0175 . 0245 . 0527 . 1260	0. 866 1. 245 1. 36 1. 82 2. 62	96. 5 89 75. 5 62. 6 54. 6	6. 4 5. 9 5. 0 4. 1 3. 6

#### B SERIES TESTS

		N=	4 Stiffener t	=0.009 \sigma_t=	73,000 $\sqrt{\frac{\overline{E}}{\sigma_s}}$	=18.88		
0. 010 . 020 . 030 . 040 . 050	11, 000 16, 500 21, 750 27, 100 32, 500	0. 0502 . 0753 . 0993 . 1238 . 1483	0.0515	0.0029 .0058 .0087 .0117 .0146	0. 0180 . 0391 . 0606 . 0822	0. 90 1. 303 1. 517 1. 643	45 43. 5 37. 8 32. 8	2. 3 2. 3 2. 0 1. 7
			Stiffener $t=0.0$	019 σ.=105,	$\sqrt{\frac{E}{\sigma_s}} = 18$	5. 75		
0. 010 . 020 . 030 . 040 . 050	29,000 35,750 43,500 52,700 64,500	0. 0921 . 1134 . 1428 . 1728 . 2095	0.0730	0.0029 .0058 .0087 . .0117 .0146	0. 0162 .0346 .0568 .0881 .1169	1. 62 1. 73 1. 89 2. 20 2. 34	162 86 63 55 47	10. 3 5. 4 4. 0 3. 4 2. 9
VIII -		St	iffener $t=0.02$	9 σ.=115,00	$\sqrt{\frac{E}{\sigma_s}} = 15.0$	)2		
0. 010 . 020 . 030 . 040 . 050	42, 500 50, 000 59, 000 68, 500 78, 700	0. 1232 . 1450 . 1680 . 1984 . 2280	0. 1078	0.0029 .0058 .0087 .1168 .1458	0. 0154 . 0314 . 0515 . 0790 . 1057	1. 54 1. 57 1. 72 1. 98 2. 11	154 78 57 49 42	10. 3 5. 1 3. 7 3. 2 2. 7
		St	iffener $t=0.049$	9 σ <sub>s</sub> =115,00	$\sqrt{\frac{E}{\sigma_s}} = 15.6$	02		
t	P	$\frac{P}{(N-1)\sigma_i}$	$\left  \begin{array}{c} N \\ \overline{N-1} A_{*t} \end{array} \right $	$\frac{1.00t}{N-1}$	Azh	W	$\frac{W}{t}$	С
0. 010 . 020 . 030 . 040 . 050	70, 000 76, 500 85, 500 98, 000 112, 500	0. 2030 . 2220 . 2480 . 2840 . 3260	0. 192	0.0033 .0066 .0099 .0133 .0166	0.008 .046 .079 .117	0.80 1.53 1.98 2.34	80 51 49 47	3, 3 3, 2 3, 1

Table III.—Computation of effective width of sheet working with stiffeners

C SERIES TESTS

		N=5 St	iffener t = 0.01	9 L=6 σ,:	=120,000 🔨	$\frac{E}{\sigma_s} = 14.71$		
t	P	$\frac{P}{(N-1)\sigma_*}$	$\left \begin{array}{c} N \\ N-1 \end{array}\right $	$\frac{0.875t}{N-1}$	A.A	W	$\frac{W'}{t}$	C
0.009 .019 .029	38, 750 43, 500 54, 800	0.0807 .0906 .1141	0.0675	0.0020 .0042 .0065	0. 0112 . 0189 . 0401	1, 248 , 995 1, 384	138 52. 4 47. 7	9. 30 3. 51 3. 23
			L=12 σ.	=102,500	$\sqrt{\frac{E}{\sigma}} = 15.90$			
0.009 .019 .029	32, 000 38, 000 47, 000	0.0780 .0926 .1145	0. 0675	0. 0020 . 0042 . 0065	0. 0085 . 0209 . 0405	0. 945 1. 10 1. 395	105 58 48	6, 6 3, 6 3, 0
•			L=18	$\sigma_i = 71,500$	$\sqrt{\frac{E}{\sigma_i}}$ =19.08			
0.009 ;019 ;029	23, 100 28, 750 35, 000	0.0808 .1005 .1224	0.0675	0.0020 .0042 .0065	0.0113 .0288 .0504	1, 255 1, 514 1, 735	139, 5 79, 6 59, 8	7. 3: 4. 1: 3. 1-

Table IV.—Column tests—individual stiffeners

[Stiffener thickness, 0.029 inch]

#### FLAT-END TESTS

Length (inches)	Sheet thickness (inch)	ρ (inch)	L! ho	Failing load (pounds)	Area ! (square inch)	Failing stress (lb./ sq. in.)
0. 97 . 94 . 89 1. 46 6. 00 9. 23 9. 22 2. 20 13. 81 13. 84 13. 82 18. 17	0, 021 . 021 . 019 . 021 . 021 . 019 . 019 . 019 . 019 . 021 . 019	0. 2247 . 2247 . 2235 . 2247 . 2235 . 2235 . 2235 . 2235 . 2235 . 2247 . 2235 . 2247 . 2235	4. 3 4. 2 4. 0 6. 5 26. 7 41. 3 41. 3 41. 2 41. 1 61. 5 62. 0 61. 5 82. 6	14, 320 14, 010 13, 845 13, 800 13, 400 10, 860 10, 150 11, 000 11, 620 9, 040 9, 170 8, 480	0. 1045 . 0965 . 1022 . 1032 . 1002 . 0946 . 0960 . 0956 . 0944 . 0885 . 0944 . 0988	137, 000 145, 300 135, 400 133, 800 133, 700 114, 700 105, 800 115, 000 122, 800 91, 700 100, 000 92, 900 87, 400
	<u>                                     </u>	KN	IFE-EDGE	TESTS 2		·
3. 48 4. 49 7. 54 7. 54 10. 72 10. 70 15. 32 15. 34 19. 97	0. 019 . 019 . 020 . 021 . 019 . 019 . 021 . 019 . 019	0. 2235 . 2235 . 2240 . 2247 . 2235 . 2235 . 2247 . 2235 . 2235	15. 6 20. 1 33. 6 33. 5 48. 0 47. 9 68. 1 68. 6 89. 4	12, 485 12, 080 9, 470 10, 240 6, 800 7, 500 4, 630 4, 870 2, 765	0. 0951 . 0959 . 0972 . 0994 . 0956 . 0946 . 0988 . 0944	131, C00 126, 000 97, 500 103, 200 71, 160 79, 300 46, 800 51, 600 28, 500

Areas were determined by weighing specimens.
 In the knife-edge tests, L includes 1.50 inches due to the knife-edges.

Table V.—Section properties of stiffeners 1/2×1/2×0.030 CLOSED SECTION STIFFENERS

Closing strip thickness (inch)	Total area (square inch)	C. G. loca- tion (inch)	Ic. g. (inch) <sup>1</sup>	ρ (inch)
0. 010	0. 0943	0. 301	0. 00437	0. 216
. 020	. 1030	. 274	. 00518	. 224
. 030	. 1118	. 251	. 00587	. 229
. 050	. 1293	. 211	. 00721	. 236

1/2×1/4 OPEN SECTION STIFFENERS

Stiffener t (inch)	Area (square) inch)	C. G. loca- tion (inch)	Ic. g. (in.)4	ρ (inch)	A/t (inch)
0. 010 . 020 . 030 . 050	0. 0286 . 0572 . 0855 . 1429	0. 333 . 333 . 333 . 333	0. 00115 . 00230 . 00343 . 00575	0. 2005 . 2005 . 2005 . 2005	2. 8575 2. 8575 2. 8575 2. 8575 2. 8575

Table VI.—Computation of slenderness ratio of working units of sheet and stiffener

	Stiffener			S	Sheet 1		Total	Total	d'= 0.333A+			ρ=			
l t	Area	Ib	ı	w	Area	Ib	area	Ib	$\frac{\frac{t}{2}A_{sh}}{A_T}$	$A_t(d')^2$	Ic.g.	$\sqrt{\frac{I}{A}}$	L	$I_{e}/\rho$	σ.
0. 0283	0. 0809	0. 01220	0. 010 . 020 . 030 . 040 . 050	1. 54 1. 57 1. 72 1. 98 2. 11	0. 0154 . 0314 . 0515 . 0790 . 1057	0. 0000005 . 0000042 . 0000155 . 0009423 . 000088	0. 0963 . 1123 . 1325 . 1601 . 1864	0. 01220 . 01220 . 01221 . 01224 . 01229	0. 2805 . 2420 . 2190 . 1780 . 1586	0. 00760 . 00660 . 00635 . 00509 . 00470	0. 0046 . 0056 . 00586 . 00715 . 00759	0. 2185 . 222 . 2225 . 211 . 2015	9. 25 9. 25 9. 25 9. 25 9. 25 9. 25	41. 5	115,000

From table IV.

Table VII.—Comparison of strength properties of corrugated sheets of 24ST aluminum alloy and stainless steel, for flat end conditions

 $L/\rho = 35$ 

Stainl	ess steel	Dı	ıral	Stress
R/l	Failing P/A	Failing P/A	ratio	
33 88 100 162 264	145, 000 30. 7 132, 000 35		48, 600 33, 200 30, 700 21, 400 13, 200	3. 55 4. 37 4. 30 4. 50 3. 48
		$L/\rho = 70$	· · · · · · · · · · · · · · · · · · ·	
33 88 100 163 264	122,000 111,000 106,000 63,000 43,000	10. 5 30. 7 35 57 92	36, 500 33, 200 30, 700 21, 400 13, 200	3. 34 3. 34 3. 46 2. 94 3. 26
		$L/\rho = 100$		
33 88 100 162 264	87, 000 86, 000 84, 000 52, 000 40, 000	10. 5 30. 7 35 57 92	27, 500 27, 500 27, 500 27, 500 21, 400 13, 200	3. 16 3. 12 3. 05 2. 43 3. 02

15

TABLE VIII.—Stainless steel test specimens

0		Area	Failing	Failing	Thie	kness	Longt	Wish	Num-		Area of	Percent	Pitch	Туре
Specifica- tion No.	Weight (lbs.)	(square inch)	load (lbs.)	stress (lb./sq. in.)	Stiff- ener (inch)	Sheet (inch)	Length (inches)	Width (inches)	ber of stiff- eners	Area of sheet	stiff- eners	force- ment	(inches)	of fail- ure
A-1a A-1b A-2a A-3a A-4a A-4a A-4a A-5a A-5a A-6a A-6a A-6b A-7b A-8a A-9a A-10a A-11a A-11b A-11a A-11b A-11a A-12a A-12a A-12a A-12a A-12b A-13a A-14b A-15a A-16a A-26a A-36a	0. 587	0. 2200 2155 1975 1975 1963 1754 1700 1238 1200 1663 1277 325 327 3255 3240 2311 2605 2365 1858 1866 3745 380 3105 3105 310 310 310 310 310 310 310 310 310 310	13, 720 14, 470 11, 470 11, 150 9, 200 85, 610 55, 870 5, 920 21, 650 18, 210 13, 120 9, 660 6, 530 16, 485 16, 940 12, 570 16, 485 16, 240 12, 570 17, 340 7, 340 7, 340 7, 340 7, 340 7, 340 7, 340 12, 570 10, 180 33, 250	62, 400 67, 250 68, 750 51, 250 55, 750 52, 490 55, 300 44, 800 66, 200 56, 200 56, 200 56, 200 56, 200 56, 200 56, 200 56, 200 56, 200 56, 200 66, 200 66, 200 66, 200 66, 200 66, 200 66, 200 66, 200 66, 200 66, 200 67, 600 67, 600 68, 200 68, 20	0.010 0.010	0.009 0.009 0.009 0.009 0.009 0.009 0.009 0.009 0.001 0.014 0.014 0.014 0.014 0.015 0.015 0.018	\$\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	8. 33 8. 32 9. 28 9. 24 9. 28 9. 24 9. 22 9. 24 12. 07 12. 07 12. 07 12. 07 13. 48 10. 88 10. 88 10. 19 10. 19 10. 67 11. 07 11. 07	55443392222776644332227755443322277554422775533222554433226644333322775544333377554433222996644332	0.0750 0.0749 0.835 0.834 0.831 0.835 0.666 0.666 1.08	0. 1450	63. 7 2 9 9 5 44. 3 8 3 3 1. 3 2 2 6 8 0. 8 8 3 1. 3 2 2 6 8 0. 5 5 2 1 4 4 4 5 2 5 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1. 86 1. 86 2. 79 4. 19 6. 6. 6. 6. 6. 6. 6. 6. 6. 6. 6. 6. 6. 6	FEFFUHFFFFAAFFHHEFFFHEHAGGAAEEEEAEEAEEDEELFFFFFFFFFFFFFFFFFFFFFFFF

 ${\bf Table\ VIII.} \color{red} \color{blue} -Stainless\ steel\ test\ specimens \color{blue} \color{blue} \color{blue} - {\bf Continued}$ 

		Area	Failing	Failing	Thic	kness			Num-		Area of	Percent		Туре
pecifica- ion No.	(lbe) (s	square inch)	load (lbs.)	stress (lb./sq. in.)	Stiff- ener (inch)	Sheet (inch)	Length (inches)	Width (inches)	ber of stiff- eners	Area of sheet	stiff- eners	rein- force- ment	Pitch (inches)	of fail ure
	(1bs.) (S) (S) (S) (S) (S) (S) (S) (S) (S) (S	0. 859 4605 4605 3605 3715 3705 3325 2725 2725 2725 2725 2725 2725 272		(lb./sq.	ener		1. (inches) 9. 203 9. 233 9. 225 9. 224 9. 229 9. 244 9. 229 9. 244 9. 229 9. 244 9. 220 9. 244 9. 220 9. 244 9. 220 9. 244 9. 220 9. 244 9. 220 9. 244 9. 220 9. 244 9. 220 9. 244 9. 220 9. 244 9. 220 9. 244 9. 220 9. 244 9. 220 9. 244 9. 220 9. 244 9. 220 9. 244 9. 220 9. 244 9. 220 9. 244 9. 220 9. 244 9. 255 9. 244 9. 255 9. 265 9. 266 9. 274 9. 267 9. 274 9. 275 9. 274 9. 275 9. 276 9. 277 9. 2	15. 77 9. 26 9. 26 9. 27 10. 55 10. 55 10. 55 10. 55 10. 26 10. 26 10. 23 10. 26 10. 26 1	stiff-		stiff-	force-		of fail

Table VIII.—Stainless steel test specimens—Continued

				Failing	Thic	kness			Num-			Percent		<b></b>
Specifica- tion No.	Weight (lbs.)	Area (square inch)	Failing load (lbs.)	stress (lb./sq. in.)	Stiff- ener (inch)	Sheet (inch)	Length (inches)	Width (inches)	ber of stiff- eners	Area of sheet	Area of stiff- eners	rein- force- ment	Pitch (inches)	Type of fail ure
A-97a	2. 502	0. 947	48, 800	51, 500	0. 030	0.048	9. 23	14. 35	3	0. 6890	0. 2580	21. 0	6. 74	î
A-97b	2. 502	. 946	51, 100	54,000	.030	. 048	9. 25	14. 37	3	. 6900	. 2560	20.9	6. 74	ĭ
A-98a	2. 958	1. 115	124,600	111,700	. 050	.010	9. 27	10.35	7	. 1035	1.0115	90. 2	1.56	1 G
A-98b	2.906	1.100	119, 200	108, 300	. 050	.010	9. 24 9. 24	10.40 11.50	7	. 1040 . 1035	. 9960 . 5785	90. 1 82. 0	1. 56 3. 51	j
A-99a	1.803	. 682	68, 700	100, 100 99, 800	. 050	.009	9. 24	11.55	4	. 1040	. 5780	82.0	3. 51	j
A-99b	1.800	. 682	68, 000 55, 040	100,000	.050	.009	9. 26	13.06	3	. 1175	. 4335	72.8	6.02	Ğ
A-100a A-100b	1.462 1.464	. 551 . 5545	54, 780	99,000	. 050	.009	9. 24	13.05	3	.1174	. 4371	72.8	6.02	Ö
A-101a	1. 004	.3795	35, 750	94, 200	. 050	.009	9. 25	10.35	2	0932	2863	63.0	9. 37	G
A-101h	1.000	.378	37, 150	98, 250	. 050	.009	9. 25	10.40	3 2 2 5	. 0936	. 2844	62. 6	9. 37	G
A-102a	2. 285	.866	100, 900	116, 400	. 050	. 013	9. 22	11. 12		. 1445	. 7215	81.6	2. 51	G
A-102b	2. 285	. 866	97, 500	112, 700	. 050	. 013	9. 22	11.05	5	. 1437	. 7223	81.6	2, 51	Ğ
A-103a	2.052	. 774	62,400	80,600	. 050	. 014	9. 26	13.88	4	. 1942	. 5798	70.6	4. 30	Ţ
A-103b	2.041	. 770	75, 910	98, 500	. 050	. 014	9. 26	13.86	4	. 1940	. 5760	70.6	4. 30 6. 69	G
A-104a	1.671	. 632	45, 800	72, 500	. 050	. 014	9. 24	14.44	3	. 2020 . 2015	. 430 . 4255	60. 5 60. 2	6.69	Ğ
A-104b	1.658	. 627	55, 050	88,000	. 050	. 014	9. 24 9. 24	14. 40 11. 04	2	. 1546	. 2854	50. 4	10.03	ă
A-105a	1. 164	. 440	37, 440	85,000	. 049	.014	9. 24	11.04	2	. 1548	2862	50.6	10.03	Ĵ
A-105b	1. 167	. 441	35, 600 113, 300	80,700	. 050	.018	9. 20	9. 87	6	. 1778	.8732	82. 2	1.76	Ĵ
A-106a A-106b	2. 762 2. 760	1. 051 1. 047	120, 100	107, 800 114, 800	. 050	.018	9. 22	9. 80	6	1765	.8705	82. 1	1.76	Ğ
A-107a	2, 700	.765	74, 650	97, 600	. 050	.018	9. 22	10.05	ı 4	. 1810	. 584	73. 0	3.01	J
A-107a	2. 024	. 766	73.070	95, 500	. 050	.018	9. 24	10.05	4	. 1810	. 585	72.9	3.01	J
A-108a	1, 665	. 6305	57, 140	90, 500	. 050	.018	9. 23	10.42	3	. 1877	. 4428	63. 7	4.69	Į Į
A-108b	1. 658	. 628	55, 860	89,000	. 050	.018	9. 23	10.43	3	. 1879	. 4401	63. 5	4. 69	Ţ
A-109a	1. 167	. 442	25,000	56,600	. 050	.018	9. 22	8.06	2	. 1452	. 2968	54.0	7.03	ĵ
A-109b	1. 153	. 437	38, 300	87,600	. 050	. 018	9. 22	8.06	2	. 1452	. 2918	53. 5	7.03	ĭ
A-110a	1. 330	. 5025	39, 720	89,000	. 050	.018	9. 25	11. 55	2	. 2080	. 2945	43.7	10. 54	1 6 6
A-110b	1. 421	. 536	43,900	81,900	. 050	.021	9. 26	11.54	2	. 2420	. 2940	39. 9	10. 54 2. 01	X
A-111a	3. 136	1. 187	129, 300	109,000	. 050	. 029	9. 24	11.04	6	. 3205	.8665	71. 2 71. 1	2.01	Y
A-111b	3. 116	1. 180	131, 300	111,300	. 050	. 029	9. 23 9. 23	11. 01	6	.3195	. 8605 . 5780	61. 5	3. 12	j
A-112a	2. 320	.879	92,900	105, 700	. 050	. 029	9. 23	10.37	1	.3010	. 5797	61. 5	3. 12	j
A-112b	2, 326	. 881	87, 300 66, 390	99,000 90,000	.050	. 029	9. 25	10.36	3	3005	. 4345	51. 5	4.68	F
A-113a A-113b	1. 945	. 735	68, 480	93, 200	. 050	. 029	9. 22	10.36	3	3003	. 4337	51.5	4.68	F
A-114a	1. 391	. 525	43,650	83,000	.050	. 029	9. 26	8.06	2	. 2340	. 2910	41.6	7.03	G
A-114b	1. 392	. 525	47, 180	90,000	. 050	. 029	9. 26	8.07	2	. 2340	. 2910	41.6	7.03	Q
A-115a	1.699	. 641	44, 820	69, 700	. 050	. 029	9. 25	11.97	2	. 3475	. 2935	31.5	10.93	G
A-115b	1.592	. 6395	45, 100	70, 800	. 050	. 029	9. 25	11.96	2	. 3470	. 2925	31.5	10.93	ų.
A-116a	3, 648	1.380	181,900	131,800	. 050	. 048	9. 24	10.38	6	. 4985	. 8815	62.0	1.87	1
A-116b	3.630	1.375	173, 200	126,000	. 050	. 048	9. 23	10.40	6	. 4995	. 8755 . 5910	61. 9 52. 2	1. 87 2. 81	FGGGGJJJ
A-117a	2. 757	1.045	116, 400	111, 400	. 050	. 048	9. 22 9. 23	9. 45 9. 44	1	4530	. 5890	52. 2	2.81	Ġ
A-117b	2. 751	1.042	117, 500 76, 100	112,800 84,200	. 050	.048	9. 23	9.50	3	.4650	. 4400	41.5	4. 22	J
A-118a A-118b	2. 385 2. 377	.900	95,000	105,600	. 050	.048	9. 22	9.48	3	4550	. 4450	42. 2	4. 22	G
A-1180 A-119a	2. 377	1. 130	65, 300	57, 800	. 050	.048	9. 24	14. 14	3	. 6790	. 4510	32.3	6.56	J
A-119h	2. 988	1. 130	80, 200	71,000	. 050	. 048	9. 25	14. 16	3	. 6795	. 4505	32.3	6. 56	J
Λ-119Λa	2. 365	. 895	52, 500	58, 700	. 050	. 049	9. 24	12. 29	2	. 6015	. 2935	21.0	11.24	G
A-119Ab	2. 359	. 895	46,000	51, 400	. 050	. 049	9. 22	12, 30	2	. 6025	. 2925	21.0	11, 24	J

TABLE IX
[Stiffener pitch=2.50 inches]

				Failing	Thick	eness			Num-			Percent	Туре
	Weight pounds)	Area (square inch)	Failing load (pounds)	stress (lb./sq. in.)	Stiff- ener (inch)	Sheet (inch)	Length (inches)	Width (inches)	ber of stiffen- ers	Area of sheet	Area of stiffeners	rein- force- ment	of failure
B-120a B-120b B-121a B-121b B-122a B-122b B-123a B-123b B-124a B-125a B-125a B-125a B-126b B-127a B-127b B-127a B-127a B-127a B-127a B-127a B-128a B-130a B-130a B-131a B-131a B-131a B-131a B-131a B-131a B-131a B-133a B-133b B-1334a B-1334a B-1334b B-1334a B-1334b B-1334a B-1334b B-1334a B-1334b B-1334a B-1334b B-1334a B-1335b	0. 492 . 484 . 714 . 709 . 946 1. 389 . 946 1. 377 . 772 . 767 . 981 . 987 1. 210 1. 210 1. 210 1. 240 1. 647 1. 649 1. 496 1. 496 1. 498 1. 772 1. 745 2. 017 2. 017 2. 017 2. 017 2. 017 2. 017 2. 018	0.1863 .1834 .2702 .2046 .3540 .3570 .5215 .5210 .2910 .2895 .3715 .3738 .4580 .4590 .4590 .4730 .4730 .4750 .5665 .5665 .5660 .7350 .6710 .6600 .7605 .8360 .8375 .7600 .8375 .7705	10, 870 10, 570 15, 950 15, 950 20, 600 21, 400 33, 100 28, 910 28, 910 28, 910 42, 100 42, 800 62, 550 61, 740 43, 920 41, 100 48, 820 60, 770 56, 800 59, 900 59, 900 60, 490 77, 230 86, 670 87, 77, 330 86, 670 87, 77, 400 87, 77, 400 87, 77, 400 88, 670 88, 67	58, 300 57, 600 59, 000 56, 900 60, 000 62, 400 99, 400 99, 400 91, 700 94, 900 92, 000 93, 300 103, 600 107, 000 100, 200 100, 200 101, 300 102, 500 103, 500 104, 800 105, 600 105, 600	0. 010 010 010 010 010 010 010 010 020 020	0. 009 . 009 . 018 . 018 . 019 . 048 . 009 . 019 . 019 . 029 . 029 . 048 . 048 . 048 . 048 . 048 . 049 . 019 . 029 . 029 . 048 . 048 . 048 . 048 . 048 . 049 . 049 . 049 . 049 . 049 . 049 . 048 . 048 . 049 . 048 . 048	9. 23 9. 22 9. 24 9. 27 9. 25 9. 25 9. 26 9. 24 9. 23 9. 26 9. 24 9. 23 9. 26 9. 22 9. 24 9. 22 9. 24 9. 25 9. 24 9. 25 9. 26 9. 26 9. 26 9. 27 9. 26 9. 26 9. 26 9. 27 9. 26 9. 26 9. 27 9. 26 9. 27 9. 26 9. 27 9. 26 9. 26 9. 27 9. 26 9. 26 9. 27 9. 28 9. 29 9. 20 9. 20	8. 40 8. 39 8. 41 8. 39 8. 40 8. 38 8. 51 8. 54 8.	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	0.0756 .0755 .1514 .1510 .2436 .2433 .4020 .4015 .0754 .0755 .1592 .1396 .2440 .4020 .0755 .0754 .1508 .2440 .4020 .0755 .0754 .1508 .2402 .0936 .0850 .1780 .1880 .1780 .1896	0.1107 1079 1188 1136 1107 1195 1195 2140 2142 2150 2245 2210 3220 3216 3220 3216 3220 3216 3220 3216 3220 3216 325 327 327 327 328 338 3350 5774 5770 5820 5770 5770 5845	55. 2 54. 5 39. 7 27. 6 28. 2 20. 0 20. 0 70. 5 70. 3 52. 8 53. 0 42. 6 42. 6 42. 6 42. 1 31. 5 78. 2 78. 1 64. 3 52. 7 52. 4 41. 0 41. 0 41. 0 41. 0 53. 0 54. 0 55. 0 56. 3 56. 3 56. 3 56. 3 56. 3 56. 3	FEFFEEDOOGFFFFFAABBFFFEEJJJFFFFJJG;

TABLE X

				F-'''	Thic	kness								
Specifica- tion no.	Weight (pounds)	Area (square inch)	Failing load (lbs.)	Failing stress (lb./sq. in.)	Stiff- ener (inch)	Shect (inch)	Length (inches)	Width (inches)	Num- ber of stiffen- ers	Area of sheet	Area of stiff- eners	Stiff- ener pitch (inches)	Percent rein- force- ment	Type of failure
C-136a C-136a C-136b C-136b C-136a C-5b C-138a C-138b C-138b C-138b C-140b C-141ab C-141ab C-141ab C-141ab C-141ab C-142a C-142b C-143aa C-144b C-145a C-145a C-145a C-145a C-146a C-146a C-146a C-146b C-146a C-150a C-151ab C-151ab C-151ab C-153a C-153a C-156b C-156ab C-156ab C-156ab C-156ab C-156ab C-166ab C-166ba C-165ba	0. 178	0. 1700 1656 1663 1670 1683 1692 1694 1684 1245 1217 1241 1228 1200 12127 1250 1228 1227 1804 1720 1698 11227 1764 1703 3210 3210 3210 3210 3210 3210 3210 32	7, 010  5, 870  6, 920  (1)  6, 940  6, 940  6, 940  6, 95, 760  5, 760  5, 760  5, 760  7, 170  9, 200  8, 870  7, 1780  6, 6210  14, 670  11, 650  11, 650  11, 650  11, 650  11, 650  11, 1880  12, 1890  13, 1890  14, 1890  15, 180  16, 180  17, 180  18,	41, 200  35, 390  35, 400  (1)  23, 600  (1)  55, 400  57, 100  44, 4800  44, 4800  44, 4800  44, 4800  44, 500  55, 500  55, 500  55, 400  55, 500  55, 600  42, 100  33, 500  34, 650  35, 300  33, 650  33, 650  34, 650  35, 400  55, 600  42, 600  62, 500  66, 200  66, 200  67, 600  68, 600  69, 700  70, 600  71, 500  71, 500  71, 500  71, 500  71, 500  71, 500  71, 500  71, 500  71, 500  71, 500  71, 500  71, 500  71, 500  71, 500  71, 500	0.010 0.010	0.009 0.009	6670526888855508684482255884748688444322885848664465254874446768444435544482258553886844822568734467674442228857344676745442228857344676745447878474787886844487778477887888548857888578	05 047 07 04 09 04 44 42 04 44 38 28 78 28 78 28 78 04 07 04 10 10 10 10 10 10 10 10 10 10 10 10 10	222222222222222222222222222222222222222	0. 1084 0. 1083 1086 1086 1086 1086 1088 1204 1204 1204 1204 1206 0666 0666 0666 0666 0666 0666 0666	0. 0616 0. 0573 0. 0573 0. 0573 0. 0573 0. 0573 0. 0573 0. 0584 0. 0600 0. 0480 0. 0580 0. 0584 0. 0581 0. 0584 0. 0581 0. 0582 0. 0865 0. 0865 0. 0865 0. 0870 0. 087	11. 16 11	23.5 3 4 5 5 7 0 7 1 0 8 3 3 2 2 3 3 3 3 2 4 4 1 9 3 3 3 2 3 3 3 3 2 3 4 4 3 3 3 3 2 3 3 3 3	A FFF G AAAFFFFGG AAAAHH GGGAAAAHAJRFFFFFFFFFFFFFGGGGGAAAAFGGGGHHHAAEEAGGGGGFEAEAAG GFFFGEE

<sup>1</sup> Too flimsy.

TABLE X-Continued

Specifica- tion no.	Weight (pounds)	Area (square inch)	Failing load (lbs.)	Failing stress (lb./sq. in.)	Stiff- ener (inch)	Sheet (inch)	Length (inches)	Width (inches)	Num- ber of stiffen- ers	Area of sheet	Area of stiff- eners	Stiff- ener pitch (inches)	Percent rein- force- ment	Type of failure
C-174a C-174b C-175a C-175b C-175b C-175b C-176b C-177b C-176b C-177b C-176b C-177b C-52a C-178b C-178b C-178b C-178b C-187b C-188b C-189b C-181b C-181b C-181b C-181b C-181b C-181b C-183b C-184c C-183b C-184c C-184b C-188b C-188b C-188b C-188b C-188b C-188b C-188b C-189b C-190b C-191a C-191b C-191a C-191b C-191a C-191b C-201b	.63 .68' 1.121 1.13 1.64' 1.64' 2.42' 2.41' 3.23' 3.23' .73' .72 1.29	3380 3845 3846 3846 3846 3846 3846 3846 3846 3846	33, 500 29, 450 33, 500 29, 780 21, 420 21, 420 25, 270 41, 930 41, 740 41, 730 41, 740 44, 730 41, 740 44, 730 41, 740 44, 730 41, 740 44, 730 41, 740 44, 730 55, 900 60, 132, 356 60, 34, 690 60, 34, 690 60, 34, 690 60, 34, 690 60, 34, 690 60, 130 60, 1	65, 100 65, 200 103, 600 101, 600 87, 600 101, 600 87, 600 101, 600 87, 600 101, 200 101, 200 1	0 .0200 0 .0300 0 .	019	13. 84 84 84 84 84 84 84 84 84 84 84 84 84	9. 30 9. 29 9. 28 9. 28 9. 29 10. 23 10. 25 10. 22 10. 23 10. 35 10.	33444444466666878554265	1672   1672   1765   1762   1765   1762   1762   1767   1764   1671   1764   1671   1672   1672   1673   1673   1673   1673   1673   1673   1674   1847	. 2209 . 2137 . 2197 . 2209 . 3264 . 3393 . 3214 . 3252 . 3263 . 3263 . 3264 . 3266 .	1.87 1.87	52. 3 59. 3 61. 8 60. 6 60. 6	

Table X—Continued

		4	Varie.	Failing	Thic	kness			Num-			Stiff-	Percent	
Specifica- tion no.	Weight (pounds)	Area (square inch)	Failing load (lbs.)	stress (lb./sq. in.)	Stiff- ener (inch)	Sheet (inch)	Length (inches)	Width (inches)	ber of stiffen- ers	Area of sheet	Area of stiff- eners	ener pitch (inches)	rein- force- ment	Type of failure
C-210h C-211a C-211b C-211a C-211a C-211a C-212a C-212a C-213a C-213a C-214b C-214a C-215a C-214a C-215a C-216b C-217a C-216b C-217a C-216b C-217a C-216b C-218h C-218h C-219a C-218h C-219a C-219a C-219a C-210a C-210a C-210a C-210a C-220a C-220a C-220a C-221a C-221a C-222a C-223a C-224a	1. 133 1. 509 1. 509 1. 509 1. 509 1. 509 1. 324 1. 304 1. 307 1. 377 1. 370 1. 647 1. 391 1. 956 1. 966 1. 996 1. 926 1. 931 1. 299 1. 956 1. 966 1. 301 1. 299 1. 956 1. 966 1. 301 1. 299 1. 956 1. 966 1. 301 1. 299 1. 942 1. 440 1. 100 1. 956 1. 100 1. 956 1. 311 1. 299 1. 311 1. 552 1. 440 1. 309 1.	. 5975 . 5900 . 5910 . 5920 . 5880 . 5900 . 4935 . 5910 . 4935 . 5925 . 7725 . 7725 . 7725 . 7750 . 7785 . 7650 . 7650	92, 000 91, 700 86, 100	104, 400 197, 000 98, 500 94, 100 82, 300 63, 800 70, 300 44, 400 94, 500 111, 400 111, 400 111, 400 111, 400 111, 400 111, 400 111, 400 111, 400 150, 600 66, 300 84, 500 77, 200 84, 500 77, 200 84, 500 77, 200 84, 500 84, 500 84, 500 84, 500 84, 500 84, 500 84, 500 84, 500 84, 500 84, 500 86, 300 86, 300 86, 300 86, 300 86, 500 87, 500 88, 900 88, 900 89, 900 80, 900 81, 900 81, 100 81,	0. 030 0.031 0.031 0.031 0.031 0.031 0.030	0.009 0.009	13. 87 18. 551 18. 568 13. 866 13. 866 14. 866 15. 866 16. 866 17. 866 18.	13. 53 13. 54 19. 32 9. 33 9. 34 9. 32 9. 33 9. 34 9. 31 12. 12 12. 12 12. 12 12. 12 12. 12 12. 12 12. 12 13. 54 13. 55 13.	222222222233333333333333333333333333333	0. 1218 1220 1220 1220 1220 1220 1220 1220	0. 1642 1615 1610 1621 1603 1621 1603 1621 1623 1623 1623 1629 1623 1623 1629 2448 2447 2447 2447 2380 2413 2424 2447 2447 2447 2380 2495 2435 2455 2455 2455 2455 2455 2455 245	12.6565 12.6565 12.6565 12.6565 12.65662 12.65662 12.65662 12.65662 12.65662 12.66662 12.666662 12.6666662 13.66663 13.33	41. 5 4. 7 3 6 6 6 7 7 7 1 2 4 6 6 6 7 7 7 1 2 4 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	АНООБОООБЕБЕБЕБЕООООБИБЕБЕВООООБЕБЕБЕВВООООБИБЕБЕВВООООБЕБЕБЕННОООООБИБЕБИНОООООБИННЕННО 100

Table XI.—Stiffeners without sheet attached

				<del>,</del>			
Specifica-	Weight	Area	Failing	Failing	Thickness		
		(square	load	stress	of stiffener	Length	$L/\rho$
tion no.	(pounds)	inch)	(pounds)	(lb./sq.in.)	(inch)	(inches)	$L/\rho$
			(р»ши»	(15,704,127)			
D-244a	0.030	0.0283	2,650	93, 700	0.011	0.50	
D-244b	. 029	. 0274	2, 420	88, 400	.011	3. 70	18. 5
D-245a	. 051	. 0276	1,660	60, 100	.011	3. 70	18. 5
D-245b	.051	.0276	1,980	71,700	.011	6. 46	32. 2
D-246a	. 073	. 0276	1, 200	43,500	.011	6. 46 9. 24	32. 2
D-246b	. 073	. 0276	1,420	51,400	.010	9. 24	46.0
D-247a	.110	.0288	835	29,000	.010	13. 87	46.1
D-247b	. 110	. 0288	900	31, 200	.010	13. 87	69. 2 69. 2
D-248a	. 155	.0292	550	18.800	.010	18. 50	92. 2
D-248b	. 151	. 0285	550	19,300	.010	18. 50	92. 2
D-249a	. 043	. 0406	3,800	93, 500	.015	3.70	18. 5
D-249b	. 043	. 0406	4,300	105, 800	.015	3. 70	18.5
D-250a	. 074	. 0400	2, 920	7, 300	.015	6.46	32. 2
D-250b	. 074	. 0400	3, 020	75, 500	.015	6, 47	32. 2
D-251a	.110	. 0415	2, 310	55,600	.015	9. 25	46. 1
D-251b	. 107	. 0405	2, 140	52,900	. 015	9. 24	46.0
D-252a	. 158	. 0398	1, 420	35, 600	.015	13.88	69. 2
D-252b	. 166	. 0434	1, 440	34, 600	. 015	13, 88	69. 2
D-253a	. 220	. 0416	1, 030	24, 800	. 015	18. 50	92. 2
D-253b	. 222	. 0419	1, 100	26, 200	. 015	18.50	92. 2
D-254a	. 055	. 052	5, 420	104, 100	. 020	3.70	18. 5
D-255a	. 096	.0518	4, 330	83, 600	,020	6.48	32. 3
D-255b	. 096	. 0519	4, 400	85,000	. 019	6. 49	32. 3
D-256a	. 141	. 0534	2,990	56,000	.020	9. 24	46.0
D-256b	. 141	. 0534	3, 240	60, 700	. 020	9. 25	46.1
D-257a	. 214	. 0540	2, 200	40, 700	. 020	13. 86	69. 2
D-257b	. 217	. 0546	2, 540	46, 500	.020	13. 89	69. 2
D-258a	. 283	. 0555	1,730	31, 200	. 020	18. 48	92. 1
D-258b	. 281	. 0553	1, 895	34, 300	.020	18. 46	92. 0
D-621a	. 081	. 077	10,470	136, 000	· .030	3. 68	18. 4
D-621b	. 081	. 0765	10, 140	132, 800	. 030	3, 70	18. 5
D-622a	. 146	. 077	6,900	90, 100	.030	6.46	32. 7
D-622b	. 146	. 077	7,300	94, 700	. 030	6, 46	32. 7
D-623a	. 213	. 0805	5,900	73, 400	. 030	9. 25	46. 1
D-623b	. 212	. 0805	5, 600	69, 600	. 030	9. 24	46. 0
D-6243	. 318	. 0800	3, 970	49, 700	. 030	13. 87	69. 2
D-624b	. 318	.0800	4, 000	50,000	. 030	13. 87	69. 2
D-625a	. 426	. <b>0</b> 805	3, 110	38, 600	. 030	18. 49	92. 1
D-625b	. 427	. 0805	3, 110	38, 600	. 030	18. 51	92. 4
D-626b	. 150	. 1425	19, 800	139, 000	. 049	3.68	18. 4
D-627a	. 265	. 143	15, 100	105, 800	. 048	6.48	<b>32</b> . 3
D-627b D-628a	. 263	. 1415	15, 600	110,000	. 048	6.48	32.3
D-628a D-628b	. 380	. 1435	13, 600	94,600	. 049	9. 26	46. 1
D-6286 D-6298	. 380	. 1440	13, 100	91,000	. 049	9. 22	45.9
D-629a D-629b	. 572	. 1444	9, 850	68, 300	. 049	13.85	69. 0
D-630a	. 571	. 1440	9, 150	63, 500	. 049	13. 88	69. 2
D-630a D-630b	. 764 . 760	. 1440	7, 880 8, 200	54, 800 57, 100	. 049	18. 50 18. 49	92. 2
					. 048		92. 1

Table XII [Stiffener width, W=0.500 inch. Stiffener pitch=2.50 inches]

										,				
Specifica-	Weight	Area	Failing	Failing	Thie	kness	Lameth	Width	Number		Area of	Stiff- ener	Percent	_
tion no.	(pounds)	(square inch)	load (pounds)	stress (lb./sq. in.)	Stiff- ener (inch)	Sheet (inch)	Length (inches)	(inches)	of stiff- eners	Area of sheet	stiff- eners	depth (D inches)	rein- force- ment	Type of failure
E A-259a E A-260a E A-260b E A-261a E A-261b E A-262b E A-262b E A-263a E A-264a E A-265a E A-265b E A-265b E A-266b	0. 262 . 511 . 515 . 763 . 762 . 415 . 415 . 827 . 822 1. 236 1. 232 . 597 . 591 1. 181	0. 1980 1927 1937 1911 1910 2108 2100 2085 2080 2298 2258 2258 2255	13, 050 10, 900 12, 000 9, 300 10, 450 11, 700 12, 100 11, 050 10, 150 10, 220 10, 100 10, 570 10, 410 10, 350 10, 050	65, 900 56, 600 62, 000 48, 600 54, 700 55, 450 57, 350 48, 650 49, 000 48, 600 46, 100 45, 900	0. 010 . 010 . 010 . 010 . 010 . 009 . 009 . 010 . 009 . 009 . 010 . 010 . 010	0. 009 . 009 . 009 . 009 . 009 . 009 . 010 . 010 . 010 . 010 . 010	4. 63 9. 27 9. 30 13. 94 13. 94 6. 88 6. 88 13. 76 13. 79 20. 68 9. 08 9. 15 18. 32	8. 42 8. 33 8. 40 8. 40 8. 49 8. 49 8. 45 8. 42 8. 34 8. 38 8. 42 8. 42	444444444444444444444444444444444444444	0. 0758 . 0750 . 0756 . 0756 . 0756 . 0764 . 0764 . 0842 . 0750 . 0834 . 0838 . 0842 . 0758	0. 1222 .1177 .1181 .1155 .1154 .1344 .1344 .1255 .1243 .1335 .1246 .1460 .1416 .1497	6. 500 . 500 . 500 . 500 . 500 . 750 . 750 . 750 . 750 . 750 . 750 1. 00	57. 5 56. 7 56. 7 56. 2 59. 8 59. 8 55. 5 55. 7 55. 5 59. 7 55. 5 62. 5	PERSONAPARARAGO ODRARARARAGO
EA-267a EA-267a EA-268a EA-268a EA-269b EA-270b EA-271a EA-271b EA-272a EA-272a EA-272b EA-273a	1. 708 1. 872 . 795 . 848 1. 718 1. 726 2. 373 2. 344 1. 057 1. 067 2. 099 2. 090 3. 200 3. 203 . 467 . 462	. 2228 . 2440 . 2460 . 2623 . 2658 . 2660 . 2443 . 2420 . 2780 . 2780 . 2785 . 2725 . 2725 . 2728 . 3523 . 3523 . 3523	8, 950 11, 500 11, 500 11, 500 11, 250 11, 250 11, 250 9, 870 9, 870 9, 870 10, 150 9, 600 10, 250 10, 000 20, 750 20, 900	45, 250 40, 150 47, 100 42, 200 43, 800 42, 250 42, 250 38, 400 39, 000 36, 550 36, 500 34, 750 35, 200 36, 850 35, 900 58, 900 59, 100	. 009 . 009 . 010 . 009 . 609 . 010 . 010 . 009 . 010 . 010 . 010 . 010 . 010 . 010 . 010	. 009 . 009 . 009 . 010 . 010 . 010 . 010 . 010 . 010 . 010 . 010 . 009 . 009 . 009 . 028 . 028	18. 33 26. 80 26. 79 11. 28 11. 28 12. 60 22. 66 33. 93 33. 88 13. 34 26. 80 26. 80 40. 20 40. 19 4. 64 4. 58	8. 39 8. 38 8. 46 8. 38 8. 35 8. 39 8. 40 8. 47 8. 38 8. 37 8. 41 8. 39 8. 33 8. 33 8. 33	444444444444444444444444444444444444444	. 0755 . 0755 . 0754 . 0761 . 0838 . 0835 . 0839 . 0840 . 0762 . 0838 . 0837 . 0759 . 0757 . 0755 . 2332 . 2335	. 1465 . 1473 . 1686 . 1699 . 1785 . 1823 . 1821 . 1603 . 1658 . 1922 . 1943 . 1976 . 1998 . 2025 . 2028 . 1191 . 1200	1. 00 1. 00 1. 00 1. 25 1. 25 1. 25 1. 25 1. 25 1. 25 1. 50 1. 50 1. 50 1. 50 1. 50 1. 50 1. 50	61. 9 62. 0 65. 2 65. 4 64. 6 64. 6 64. 6 64. 8 65. 8 66. 0 68. 7 68. 7 69. 2 29. 9 30. 0	FOGAAAFOGFFGGGGEE

#### TABLE XII—Continued

[Stiffener width, W=0.500 inch. Stiffener pitch=2.50 inches]

	1		· · · · · ·	Stiffener wi		0.000	1		<u> </u>					
Specifica-	Weight	Area	Failing	Failing	Thic	ness	Length	Width	Number of stiff-	Area of	Area of stiff-	Stiff- ener depth	Percent rein-	Type of
tion no.	(pounds)	(square inch)	load (pounds)	stress (lb./sq. in.)	Stiff- ener (inch)	Sheet (inch)	(inches)	(inches)	eners	sheet	eners	(D inches)	forced	failure
E 4 9750	0. 937	0. 3523	20,700	58, 750	0. 010	0. 028	9. 30	8, 38	4	0, 2346	0. 1177	0. 500	29. 6 27. 3	
EA-275b EA-276a EA-276b	. 934 1. 386	. 3510	21, 350 18, 750 19, 950	60, 800 54, 000 57, 000	. 010 . 010 . 010	. 028 . 028 . 028	9, 30 13, 96 13, 90 6, 88 6, 88 13, 78 13, 78 20, 70 20, 69	8. 35 8. 35 8. 38	4 4 4	. 2422 . 2338 . 2346 . 2355	0. 1177 . 1088 . 1132 . 1149 . 1364 . 1262 1265 . 1232 . 1238 . 1235 . 1458	. 500 . 500 . 500	28.8 29.1	0 0 0
EA-277a EA-277b	. 731 . 730	. 3495 . 3719 . 3710 . 3710	20, 850 19, 350 20, 000	56, 200 52, 150 54, 400	. 009	. 028 . 029 . 029	6. 88 6. 88 13. 78	8. 41 8. 44 8. 43	4 4 4	. 2448	. 1364 . 1262 1265	. 750 . 750 . 750	32. 8 30. 4 30. 4	E E
EA-278b EA-279a EA-279b	1. 450 2. 181 2. 170	. 3677 . 3680			. 009 . 009 . 009	. 029 . 029 . 029	13. 78 20. 70 20. 69	8. 43 8. 42 8. 43	4 4 4	. 2445 . 2442 . 2445	. 1232 . 1238 . 1235	.750 .750 .750	29. 8 30. 0 29. 8	A G
EA-280a EA-280b EA-281a	. 999 . 977 1. 968	.3815 .3730 .3746	18, 140 16, 080 15, 900	49, 800 47, 600 43, 100 42, 400	. 010 . 010 . 009	. 028 . 028 . 028	9. 15 9. 16 18. 35	8. 42 8. 33 8. 38	4 4	. 2357 . 2332 . 2346	. 1458 . 1398 . 1400	1.000 1.000 1.000	29. 8 34. 3 33. 4 33. 4	G H H A
EA-281b EA-282a EA-282b	1. 993 2. 862 3. 174	.3677 .3680 .3680 .3815 .3730 .3746 .3810 .3750 .4140	17, 750 15, 510 17, 800	40 000	. 009 . 009 . 009	. 028 . 028 . 028 . 028 . 028 . 031 . 028	18. 27 26. 70 26. 80	8. 38 8. 41 8. 50	4 4	. 2346 . 2355 . 2635	. 1464 . 1395 . 1505	1.000 1.000 1.000	34. 4 33. 3 32. 7 37. 0	A A H
EA-283a EA-283b EA-284a	1, 389 . 731 1, 463 1, 463 1, 463 1, 463 1, 463 1, 999 . 977 1, 968 1, 993 2, 862 3, 174 1, 291 1, 276 2, 572 2, 729 3, 873 3, 850 1, 695 3, 391 3, 346 5, 5, 5, 5, 5, 5	.3985 .3950 .3978	18, 350 18, 140 16, 080 15, 900 17, 750 15, 510 17, 800 17, 100 18, 050 16, 750 16, 300 15, 150 16, 050 18, 150	40, 600 41, 400 43, 000 42, 950 45, 700 42, 100 38, 600 37, 900 40, 400	. 009	. 028 . 028 . 028 . 030	9. 15 9. 16 18. 35 18. 27 26. 70 26. 80 11. 31 11. 28 22. 58 22. 60 33. 88	8.33 8.35 8.33 8.44 8.44 8.42 8.33 8.34 8.33 8.34 8.45 8.47 8.47 8.47 8.47	4 4	. 2343 . 2355 . 2372 . 2526 . 2335 . 2363 . 2450	. 1642 . 1595 . 1606	500 500 750 750 750 750 750 750 1.000 1.000 1.000 1.250 1.250 1.250 1.250	36. 3 36. 5 36. 1	cç
EA-284b EA-285a EA-285b	2. 729 3. 873 3. 850	. 4220 . 3995 . 3970	16, 300 15, 150 16, 050	38, 600 37, 900 40, 400	. 010 . 009 . 009	. 030 . 028 . 028 . 029	33. 88 33. 90	8. 34 8. 44 8. 46	4 4 4	. 2335 . 2363 . 2450	. 1660 . 1607	1. 250 1. 250 1. 250 1. 500	37. 2 36. 6 40. 4	Ğ E I
EA-286a EA-286b EA-287a	1. 695 1. 695 3. 391	. 4425 . 4425 . 4420	18, 150 18, 350 17, 800		.010 .010 .010 .010	. 029 . 029 . 028 . 028	33. 90 13. 38 13. 38 26. 81 26. 70 40. 22 40. 22 4. 64 4. 63	8. 46 8. 45 8. 45	4 4	. 2455 . 2365 . 2365	. 1970 . 2055 . 2015	1, 500 1, 500 1, 500	40. 4 42. 3 41. 8	Ğ G I
EA-288a EA-288b	5. 037 5. 094	. 4370 . 4420 . 5320	14, 000 16, 200 26, 750	32, 000 36, 600 50, 250	.009	. 029	40. 22 40. 22 40. 22 4. 64	8. 46 8. 45 8. 45 8. 40 8. 43 8. 38 8. 44 8. 43 8. 37	4 4	. 2435 . 2615 . 4105	. 1935 . 1805 . 1215	1. 500 1. 500 . 500	40. 0 36. 8 19. 9	H G H
EA-289b EA-290a EA-290b	. 706 . 705 1. 400 1. 395	. 5320 . 5270 . 5260	29, 430 27, 090 30, 400	55, 250 51, 400 57, 750	.010 .010 .010	. 049 . 049 . 049	4. 63 9. 28 9. 26	8. 44 8. 43 8. 37	4 4	. 4140 . 4130 . 4100	. 1180 . 1140 . 1160	. 500 . 500 . 500	19. 4 18. 9 19. 2	H G A
EA-291a EA-291b EA-292a	2, 090 2, 101 1, 054	. 5255 . 5255 . 5430	29, 750 27, 580 26, 650	56, 600 52, 500 49, 100	.010 .010 .009	. 049 . 049 . 050	9, 28 9, 26 13, 90 13, 96 6, 79 6, 87 13, 78	i 8.39	4 4	. 4110 . 4110 . 4180	. 1145 . 1145 . 1250	. 500 . 500 . 750	18. 9 18. 9 20. 1 21. 4	G H
EA-292b EA-293a EA-293b	1. 067 2. 136 2. 130	. 5430 . 5410 . 5380	30, 475 31, 760 26, 410	56, 200 58, 700 49, 100	.010 .010 .009 .010 .009 .009 .009	. 029 . 031 . 049 . 049 . 049 . 049 . 050 . 049 . 050 . 049 . 049 . 049	6. 87 13. 78 13. 83	8. 37 8. 38 8. 37	4 4	.4100 .4190 .4100 .4115	. 1330 . 1220 . 1280	. 750 . 750 . 750	19. 6 20. 7 20. 8	G H
E A - 275a E A - 275a E A - 275a E A - 276a E A - 276a E A - 276a E A - 277a E A - 277a E A - 278a E A - 278a E A - 278a E A - 280b E A - 281a E A - 282a E A - 282a E A - 283a E A - 283a E A - 283a E A - 283a E A - 283b E A - 283b E A - 286b E A - 287b E A - 287b E A - 289a E A - 289a E A - 290a E A - 300b E A - 300b E A - 300b E A - 300b E A - 300a E A - 30a E A - 3	3. 199 3. 213 1. 453	. 5400 . 5425 . 5540	18, 350 17, 800 17, 700 14, 000 16, 200 26, 750 29, 430 27, 090 30, 400 29, 750 30, 475 30, 475 31, 550 30, 475 31, 550 32, 000 32, 00	41, 500 40, 250 40, 400 32, 000 50, 250 51, 400 57, 750 56, 600 58, 700 49, 100 58, 700 59, 000 33, 300 50, 850 43, 800 44, 650 46, 850 47, 000 41, 550 39, 000 42, 650 48, 400 41, 550 39, 000 42, 650 48, 400 41, 550 39, 000 42, 750 48, 400 41, 550 39, 000 41, 550 39, 000 41, 550 39, 000 41, 550 39, 000 41, 550 39, 000 41, 550 41, 150 41, 150 41, 150 42, 750 41, 150 41, 150 42, 750 41, 150 42, 750 43, 800 44, 750 45, 800 46, 850 47, 850 48, 850 48, 850 48, 850 48, 850 88, 900 88, 900	.009	. 049 . 049 . 048 . 049	13. 78 13. 83 20. 70 20. 71 9. 18 9. 18 18. 33 18. 32	8.39 8.37 8.38 8.37 8.40 8.35 8.44 8.35 8.48 8.38 8.38 8.38 8.48 8.41 8.40 8.41 8.41 8.41 8.42 8.42 8.42 8.43 8.43 8.43 8.44 8.43 8.44 8.43 8.44 8.44	4 4 4 4	.4110 .4010 .4140	. 1458 . 1398 . 1400 . 1464 . 1395 . 1505 . 1606 . 1697 . 1970 . 1970 . 1970 . 1970 . 1970 . 1970 . 1970 . 1970 . 1970 . 1970 . 1970 . 1970 . 1970 . 1970 . 1970 . 1970 . 1970 . 1970 . 1970 . 1935 .	1. 500 1. 500 1. 500 1. 500 500 500 500 500 500 500 500 750 750	21. 2 24. 2 22. 9	AAAHCCCJGEIIGIHGHHGAHGHHOHDDHHIIEEHHAFIIIIIDDGGFFG
EA-295b EA-296a EA-296b	1. 469 2. 922 2. 912 4. 419	. 5565 . 5550	24, 400 19, 700	43, 800 35, 500 53, 100	. 009 . 009 . 009 . 009	.049 .049 .052	18. 33 18. 32 26. 78	8. 40 8. 35 8. 42	4 4	4115	. 1450 . 1460 . 1380	1.000 1.000 1.000		I I E
EA-297b EA-298a EA-298b	4. 520 1. 856 1. 852	. 5900 . 5750 . 5740	30, 015 28, 050 29, 500	50, 850 48, 750 51, 400	.011 .009 .009	. 051 . 049 . 049	26. 78 26. 78 11. 28 11. 27	8.38 8.38 8.38	4 4 4	. 4265 . 4105 . 4105	. 1635 . 1645 . 1635	1. 000 1. 250 1. 250	22. 8 22. 9 21. 0 24. 3 25. 1 25. 0 24. 8	H
EA-299a EA-299b EA-300a	3. 730 3. 748 5. 647	. 5770 . 5800 . 5815	24, 620 27, 175 27, 350	42, 650 46, 850 47, 000	.009 .009 .009	. 049 . 049 . 049	22. 58 22. 59 33. 90 33. 88	8. 48 8. 50 8. 46	4 4	. 4150 . 4160 . 4145	. 1620 . 1640 . 1670	1. 250 1. 250 1. 250 1. 250	25. 4	F
EA-300b EA-301a EA-301b	5. 639 2. 364 2. 365	. 5810 . 6170 . 6190	24, 150 24, 060 26, 400	41, 550 39, 000 42, 650	. 009 . 011 . 010	. 049 . 049 . 049	13. 39	8. 38 8. 41 8. 36	4 4	.4090 .4380 .4265 .4105 .4105 .4160 .4145 .4100 .4095 .4120 .4095 .4125 .4210 .0756	. 1710 . 2050 . 2095	1. 500 1. 500	25. 9 29. 4 29. 9 29. 4	į
E A-302a E A-302b E A-303a	4. 715 4. 629 6. 885	. 6160 . 6040 . 5970	29, 790 29, 000 25, 540	48, 400 48, 000 42, 750	.010 .009 .009 .010	. 049 . 049 . 049 . 050	26. 80 26. 80 40. 27 40. 20	8. 36 8. 42 8. 42	4 4 4	. 4095 . 4125 . 4210	. 1945 . 1845	1. 500 1. 500 1. 500 1. 500	28. 5 27. 4	I D D
EA-303b EA-304s EA-304b	6. 971 . 533 . 525 1. 044	. 4025	49, 460 46, 190	122, 800 115, 900	.030	.009		8. 40 8. 41 8. 39	4 4	. 0756 . 0757 . 0755 . 0752	. 3269 . 3233 . 3175	. 500 . 500 . 500	78. 5 78. 2 78. 0	G F
EA-305b EA-306a	1. 070 1. 598 1. 613	. 3985 . 3950 . 3978 . 4220 . 3978 . 4425 . 4425 . 4425 . 4380 . 4370 . 5320 . 5255 . 5255 . 5430 . 5410 . 5505 . 5550 . 5550 . 5555 . 5560 . 55760 . 5760 . 5770 . 5815 . 5815 . 5815 . 5815 . 5815 . 5815 . 5816 . 6100 . 6100	41, 400 33, 775	l <b></b>	. 030 . 030 . 030	.009	4. 60 9. 30 9. 29 13. 88 13. 94	8. 36 8. 33 8. 36	4 4	. 0749	. 3278 . 3271 . 3293	. 500 . 500	1 78.5	G
EA-3070 EA-3070 EA-3080 EA-3080 EA-3090 EA-3090	.917 .919 1.841	. 4670 . 4673 . 4660	58, 450 59, 250 52, 340	125, 300 126, 800 112, 500	. 030	. 010 . 010 . 010	6. 87 6. 87 13. 80	8. 44 8. 48 8. 49	4 4	. 0844 . 0848 . 0849	. 3826 . 3825 . 3811	. 750 . 750 . 750 . 750 . 750	79.3 79.3 79.2	
E A-308b E A-309a E A-309b	1. 804 2. 740 2. 757	. 4575 . 4620 . 4650	48, 070 40, 000 41, 690 60, 750	105, 100 86, 500 89, 800	. 030 . 030 . 030 . 030	. 009	13. 78 20. 68 20. 70	8. 42 8. 42 8. 42	4 4	. 0757 . 0842 . 0757 . 0840	. 3818 . 3778 . 3893	. 750 . 750 . 750 1. 000	79. 3 79. 2 81. 0 79. 0 81. 1	G
EA-310b EA-311a	1. 378 1. 372 2. 759	. 5265 . 5240 . 5260	59, 720 46, 450	115, 300 114, 000 88, 250	.030	.010 .010 .009	9. 15 9. 15 18. 34	8. 40 8. 38 8. 38 8. 38	4 4 4	. 0838 . 0754 . 0754	. 4402 . 4506	1.000	81. 5 81. 4 83. 4 83. 3 81. 6	Î
EA-3110 EA-312a EA-312b	2. 741 4. 083 4. 107	. 5220 . 5325 . 5355 . 5810	43, 250 42, 630 45, 580 60, 590	80, 000 85, 100 104 300	.030	.010	18. 34 18. 33 26. 79 26. 79 11. 29	8. 43 8. 37 8. 42	4 4	. 0843 . 0753 . 0758	. 4482 . 4502 . 5052	1. 000 1. 000 1. 250	81. 6 83. 3 84. 8	GC
EA-313a EA-313b EA-314a EA-314b	1.877 1.877 3.740 3.750	. 5830 . 5790 . 5815	57, 190 56, 430 56, 820	98, 100 97, 450 97, 600	. 029 . 030 . 030	.009	11. 26 22. 58 22. 53 33. 92	8. 43 8. 43 8. 44	4 4	. 0759 . 0759 . 0760	.4425 .4402 .4506 .4466 .4482 .4502 .5052 .5071 .5031 .5040 .5794 .5793 .4796	1. 000 1. 000 1. 000 1. 000 1. 250 1. 250 1. 250 1. 250 1. 250 1. 250	84. 8 84. 2 85. 1	CCC
EA-315a EA-315b EA-316a	5. 631 5. 628 2. 518	. 5800 . 5800 . 6575	42, 590 44, 920 51, 620	73, 500 77, 500 78, 500	. 030 . 029 . 031	.009	13. 88	8. 43 8. 44 8. 49	4 4	. 0759 . 0760 . 0849	. 5041 . 5040 . 5726	1. 250 1. 250 1. 500	84. 8 84. 8 85. 2	Î G
EA-316b	2. 514 5. 015 5. 035	. 6550 . 6550 . 6560	55, 300 52, 460 50, 250	84, 400 80, 100 76, 500	. 030 . 031 . 031	.009	13. 39 26. 82 26. 82	8. 40 8. 42 8. 49	4	. 0757	. 5794	1, 500 1, 500 1, 500 1, 500	86. 5 86. 5	I I
EA-317b EA-318a EA-318b EA-319a EA-319b	7. 601 7. 594 . 733	. 6605 . 6595 . 5520	40, 410 61, 050	125, 300 126, 800 112, 500 112, 500 115, 100 86, 500 88, 800 115, 300 114, 000 88, 230 80, 000 104, 300 98, 100 97, 450 97, 450 97, 500 78, 500 78, 500 67, 000 61, 200 110, 600 110, 600 190, 700 98, 950	. 031 . 031 . 030	.009 .009 .028	40. 24 4. 64	8. 40 8. 47 8. 40 8. 39	4	. 0762	. 5849 . 5833 . 3167 . 3190	.500	86. 7 53. 1 53. 2	G B B
EA-319b EA-320a	. 735 1. 467	. 5540	54, 500	98, 950	. 030	.029		8. 39	4	. 2435	.3075	.500	51. 5	Ğ

TABLE XII—Continued

[Stiffener width, W=0.500 inch. Stiffener pitch=2.50 inches]

				[Stiffener w		0.000		- Inte	11-2.50 11	icitesj				
9	377.:-1.4	Area	Failing	Failing	Thic	kness	7 41	TT7: 141	Number		Area of	Stiff- ener	Percent	Туре
Specifica- tion no.	Weight (pounds)	/equore		stress	Stiff- ener	Sheet (inch)	Length (inches)	Width (inches)	of stiff- eners	Area of sheet	stiff- eners	depth (D inches)	rein- force- ment	of failure
EA-320b EA-321b EA-321a EA-322a EA-322a EA-322b EA-325b EA-325b EA-325b EA-325b EA-327b EA-328b EA-328b EA-328b EA-332a EA-333a EA-333a EA-333a EA-333a EA-333b EA-333b EA-335a EA-335a EA-335a EA-336a EA-337a EA-337a EA-337a EA-338b EA-337a EA-338b EA-338b EA-338b EA-338b EA-338b EA-338b EA-338b EA-338b EA-338b EA-348b EA-356a EA-356b EA-36b EA	1. 475 2. 200 2. 193 1. 221 1. 255 2. 514 2. 4675 3. 767 1. 776 3. 578 3. 562 2. 400 2. 403 4. 892 2. 400 2. 403 4. 892 2. 400 2. 405 3. 102 3. 121 6. 251 9. 417 9. 430 3. 102 9. 430 9	. 9620 . 9630 . 9590 . 9620 . 9740 1. 0600	55, 540 44, 910 35, 900 68, 700 68, 700 67, 000 73, 850 70, 750 60, 510 55, 500 60, 510 571, 550 71, 550 71, 550 70, 710 66, 610 71, 550 71, 550 70, 410 71, 550 71, 550 71, 550 70, 410 71, 550 71, 550 70, 410 71, 550 71, 550 71, 550 71, 550 71, 550 71, 550 71, 550 68, 910 68, 690 78, 780 89, 100 78, 290 70, 660 89, 100 75, 500 89, 100 75, 500 89, 100 77, 410 78, 950 77, 900 77, 9	100, 300 81, 500 110, 600 110, 300 81, 500 110, 600 102, 300 64, 050 97, 800 107, 800 103, 700 96, 500 88, 500 88, 700 98, 700 98, 700 98, 500 108, 600 109, 600 109, 600 109, 600 109, 900 110, 600 110,	ener (inch)  0. 030 0.		9. 31 13. 94 13. 94 16. 87 6. 88 6. 87 9. 06 13. 78 20. 70 20. 67 9. 06 18. 32 22. 57 23. 89 11. 30 22. 25 27 20. 70 20.	8.8.446 8.846 8.846 8.846 8.846 8.846 8.846 8.846 8.846 8.846 8.846 8.846 8.846 8.846 8.846 8.846 8.84		0. 2346	0. 3199	100   100	53. 3 51. 5 53. 1 5 56. 6 6 6 4 6 6 6 4 6 6 6 4 4 6 6 6 6 6 6	0011 0111111111111111111111111111111111

#### TABLE XII—Continued

[Stiffener width, W=0.500 inch. Stiffener pitch=2.50 inches]

Specifien	of failure
tion no. (pounds) (inch) (pounds) (lb./sq. in.) (lb./sq. in.) (still ener (inch) (inches) (in	failure
$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	100000000000000000000000000000000000000
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
EA-366b         3. 353         8390         69, 500         82, 900         .050         .030         13. 97         8. 52         4         .2557         .5833         .500         66.           EA-367a         1. 805         .9200         119, 200         129, 600         .050         .029         6. 85         8. 51         4         .2463         .6682         .750         69.           EA-368a         3. 633         .9210         104, 600         113, 500         .050         .029         13. 79         8. 50         4         .2463         .6682         .750         69.           EA-368a         3. 627         .9200         103, 300         112, 400         .050         .029         13. 79         8. 50         4         .2463         .6682         .750         69.           EA-368b         3. 627         .9200         103, 300         112, 400         .050         .029         13. 79         8. 50         4         .2463         .6735         .750         69.           EA-369a         5. 463         .9225         81, 900         .88, 900         .050         .029         20. 70         8. 58         4         .2487         .6733         .750         69. <td>000000000000000000000000000000000000000</td>	000000000000000000000000000000000000000
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	0 0 0 0
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	G G G
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	Ğ
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	Ģ
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	ilä
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	ğ
EA-372a 7, 830 1, 0210 94, 700 92, 700 049 029 26, 79 8, 54 4 2470 1,000 72 FA-372b 7, 738 1,000 105, 200 104, 200 048 028 26, 80 8, 54 4 2390 7,700 1,000 72	g
	Ğ
EA-373a 3,702 1.1470 147,400 128,600 .050 .031 11.28 8.64 4 .2678 8792 1.250 74	ļĮĮ
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	) j
EA-3/4b 7.416 1.1480 134,000 116,600 050 031 22.59 8.56 4 2655 8825 1.250 74	Į Į
EA-374b 7.416 1.1480 134,000 116,600 050 031 22.59 8.56 4 2665 8825 1.250 74. EA-375a 11.120 1.1470 107,500 93,700 050 031 33.87 8.50 4 2635 8835 1.250 74. EA-375b 11.153 1.1500 105,500 91,700 049 031 33.88 8.60 4 2665 8835 1.250 74.	il ä
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	) Į
EA-376b     4.683     1.2210     143,300     117,300     .050     .029     13.39     8.52     4     .2470     .9740     1.500     77.       EA-377a     9.372     1.2220     125,200     102,400     .049     .030     26.80     8.50     4     .2550     .9670     1.500     76.	il ii
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	i G
EA-3788 14.045 1.2225 108,100 88,400 .049 .029 40.15 8.54 4 .2475 .9750 1.500 74. EA-378b 14.240 1.2380 114,900 92,800 .049 .031 40.17 8.59 4 .2660 .9720 1.500 75.	,
EA-3789 1.312 9890 123,400 124,800 050 049 4.64 5.50 4.185 5705 500 53.	i ĝ
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	1 6
EA-380b 2, 649 9910 103,000 103,800 050 049 9.34 8.52 4 4175 5735 500 54	j: ğ
EA-381a 3.937 9860 83,300 84,500 050 049 13.66 8.52 4 4175 5685 500 53.	1 8
EA-380b 3.957 .9860 83,300 84,500 .050 .049 13.56 8.52 4 .4175 .5685 .500 54.  EA-381b 3.953 .9940 84,100 84,600 .050 .049 13.90 8.54 4 .4185 .5755 .500 54.  EA-382a 2.188 1.1000 138,200 125,600 .050 .050 .689 8.48 4 .4240 .6760 .750 57.  EA-382b 2.140 1.0880 136,250 125,200 .050 .049 6.88 8.51 4 .4165 .6715 .750 57.	j j
EA-382b 2 140 1 0880 136, 250 125, 200 050 049 6 88 8 51 4 4165 6715 750 57.	3 6
EA-383a 4.300 1.0920 122,100 111,800 .050 0.49 13.78 8.52 4 .4175 .6745 .750 57. EA-383b 4.284 1.0880 118,650 109,000 .050 0.49 13.77 8.56 4 .4195 .6685 .750 57.	i j
EA-384a 6.451 1.0900 87,200 80,000 .050 .049 20.68 8.55 4 .4195 .6705 .750 57.	/ J
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	i J
EA-385b 3.081 1.1800 162,500 137,600 .050 .049 9.13 8.45 4 .4140 .7660 1.000 61	기 부
EA-3866 6.284 1.1990 120, 300 100, 300 .050 .049 18.34 8.53 4 4180 .7810 1.000 61. EA-3866 6.280 1.1980 134, 000 111, 800 .049 .049 18.33 8.53 4 4180 .7800 1.000 61. EA-3867 9.114 1.1930 105, 100 88, 000 .049 .049 2.672 8.64 4 4230 .7700 1.000 61.	i j
EA-3800	i G
EA.387b   9,226   1,230   109,300   90,900   0,50   0,51   26.79   8.48   4   4,150   7880   1,000   60.   17.4.387b   9,226   4,181   1,960   146,200   112,900   0,50   0,49   11,28   8.63   4   4,225   8735   1,250   64.	í j
EA-388b 4.195 1.2980 154,300 118,800 0.55 0.49 11.29 8.62 4 4220 8760 1.250 64	i   Î
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	2 J
EA-3890a 12.744 1.3160 122,500 93,200 0.50 0.49 33,91 8.93 4 4.375 8.785 1.250 64.	2 G
EA-390b 12.644 1.3050 106,700 81,7 .050 .049 33.91 8.64 4 .4230 .8820 1.250 64.  EA-391b 5 381 1.4020 149.50 106,800 .050 .050 13.42 8.58 4 .4290 .9730 1.500 66.	í li
EA-391b 5.385 1.4000 164,800 117,700 050 049 13.37 8.53 4 4.1180 9820 1.500 66.	Z I
EA-390b 12.644 1.3050 106,700 81,7 050 049 33.91 8.64 4 4220 .8820 1.250 64. EA-391b 5.381 1.4020 149,50 106,800 .050 .050 13.42 8.58 4 4220 .9730 1.500 66. EA-391b 5.355 1.4000 164,800 117,700 .050 049 13.37 8.53 4 4180 .9820 1.500 66. EA-392a 10.700 1.3970 150,300 107,600 .050 049 2.6.70 8.54 4 4185 .9785 1.500 66. EA-392b 10.709 1.4030 133,300 95,000 .049 .050 22.70 8.48 4 4240 .9790 1.500 67.	3 J
EA-381a 3.937 .9860 83,300 84,500 .050 .049 13.56 8.52 4 4.175 .5685 .500 53. EA-381b 3.937 .9860 138,200 125,600 .050 .049 13.90 8.54 4 4185 .5755 .500 54. EA-382b 2.140 1.0880 138,250 125,600 .050 .050 .049 13.78 8.52 4 4.185 .7755 .575 .575 .575 .6745 .750 .575 .750 .575 .750 .575 .750 .575 .750 .575 .750 .575 .750 .575 .750 .575 .750 .575 .750 .575 .750 .750	i j
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	7 J

TABLE XIII

[Stiffener pitch=2.50 inches. Stiffener depth, D=0.500 inch]

										nchj				
Specifica-	Weight	Area	Failing	Failing	Thic	kness	Towark	3077345	Num-		Area of	Stiffener	Percent	
tion no.	Weight (pounds)	(square inch)	load (pounds)	stress	Stiff- ener (inch)	Sheet (inch)	Length (inches)	Width (inches)	ber of stiff- eners	Area of sheet	Area of stiff- eners	width (W- inches)	rein- force- ment	Type of failure
E H-3944 E H-3944 E H-3954 E H-3956 E H-3956 E H-3956 E H-3957 E H-3957 E H-3958 E H-3956 E H-3956 E H-3956 E H-4006 E H	0. 264	0. 1998 1998 1997 2020 2015 2025 2120 2025 2120 2128 2125 2126 2140 2426 2410 2426 2420 2425 2330 2405 2375 3700 3740 3740 3740 3740 3740 3740 3740	12, 300 12, 500 12, 500 11, 380 11, 250 11, 250 11, 250 11, 250 11, 250 10, 630 10, 330 10, 650 10, 330 10, 650 11, 2950 11, 2950 10, 450 10, 130 10, 130 10, 150 11, 280 11,	61, 550 62, 500 52, 450 56, 350 49, 400 55, 600 50, 500 50, 500 50, 500 40, 250 40, 250 40, 250 40, 250 40, 250 40, 250 40, 250 40, 250 61, 100 51, 100 51, 100 51, 100 61, 000 61, 000 61, 000 61, 100 61, 100 61, 250 60, 25	0.009 0.010 0.010 0.010 0.010 0.009 0.009 0.009 0.010	0.009 0.010 0.009 0.010 0.009 0.009 0.010 0.010 0.009 0.010 0.009	4. 62 4. 62 4. 62 9. 25 13. 82 9. 18 13. 37 13. 82 14. 60 15. 18 16. 18 17. 70 18. 18. 18 19. 18	8. 666 667 668 88 88 88 88 88 88 88 88 88 88 88 88	***************************************	0. 0776 0. 8776 0. 8871 0. 8866 0. 8864 0. 8865 0. 8866 0. 8822 0. 8866 0. 8828 0. 9812 0. 8821 0. 882	0. 1222 .1127 .1131 .1240 .1151 .1328 .1243 .1264 .1254 .1264 .1264 .1264 .1264 .1264 .1264 .1264 .1264 .1264 .1264 .1264 .1353 .1449 .1456 .1456 .1456 .1456 .1456 .1466 .1468 .1468 .1468 .1469 .1478 .1489 .148	0.75 .775 .775 .775 .775 .775 .775 .775	57. 6 0 1 53. 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	FFAAGGCHAAGGFFAGGHFGGGGEEHHDCEECHE EECCCCCEECCEEHHAAHHCCCDGCCCCGGCCCCCC GGCGGGHGGGGGGCCCCCCCCCC

#### TABLE XIII—Continued

[Stiffener pitch=2.50 inches. Stiffener depth, D=0.500 inch]

		T	i — —	1	T	,	•				·		<del>,</del>		
	Specifica- tion no.	Weight (pounds)	Area (square inch)		Failing stress (lb./sq.in.)	Stiff-	kness Sheet	Length (inches)	Width (inches)	Num- ber of stiff-	Area of sheet	Area of stiff- eners	Stiffener width (W-	Percent rein- force-	Type of failure
_						(inch)	(inch)			eders			inches)	ment	
	E B - 410a b E B -	1. 298 1. 298 1. 298 1. 933 1. 933 1. 933 1. 802 1. 612 1. 625 2. 414 2. 421 1. 625 2. 451 1. 678 2. 545 1. 625 2. 451 1. 678 2. 555 1. 732 1. 625 1. 625 1.	0. 5280	44, 086 45, 430 38, 930 66, 130 65, 130 65, 1830 50, 632 64, 6130 65, 620 64, 6130 62, 190 65, 170 71, 670 65, 230 68, 180 68,	83, 400 85, 700 72, 650 78, 600 109, 300 97, 900 97, 900 97, 900 97, 400 83, 250 101, 300 101, 700 103, 300 104, 700 105, 200 105, 200 106, 200 107, 400 88, 500 106, 800 107, 800 108, 800 109, 700 109, 700 100, 100 100,	ener	Sheet (inch)   O. 039	8. 60 8. 55 12. 98 12. 28 13. 85 14. 60 9. 20 13. 78 14. 40 13. 33 13. 30 14. 30 15. 44 16. 40 16. 40 17. 40 18. 86 18. 80 19. 31 19. 31 1	9. 36 9. 34 9. 34 9. 34 9. 34 9. 34 9. 34 8. 66 8. 69 8. 88 8. 89 9. 90 9.	80000000000000000000000000000000000000	0. 0843 .0940 .2508 .2508 .2508 .2509 .2509 .2509 .2580 .2580 .2580 .2580 .2580 .2580 .2580 .2742 .2742 .2950 .2820 .2823 .282	0. 4437 4360 4392 4410 3542 3555 3513 3790 3721 38820 3794 4154 4254 4254 4408 5128 4078 3930 4425 4408 3515 3515 3450 3540 3535 3450 3540 3555 3450 3540 3555 3450 3540 3555 3450 3540 3555 3450 3540 3555 3450 3540 3555 3450 3540 3555 3450 3540 3555 3450 3540 3571 3745 3771 3780 4030 4030 4030 40407 4060 4110 54145 5620 5711 6713 6714 6896 6715 6896 6715 6896 6715 6896 6715 6896 6711 6713 6714 7241 7241 7241 7241 7241 7241 7241 7	1.50 1.50 1.50 1.50 1.50 1.50 1.00 1.00	83.2 4 4 6 6 5 5 5 5 6 6 9 1 7 0 0 6 9 2 2 2 4 6 6 7 0 7 7 0 0 9 6 9 5 9 6 7 7 0 6 9 6 9 5 9 6 7 7 0 6 9 6 9 5 9 6 7 7 0 6 9 6 9 6 9 6 9 6 9 6 9 6 9 6 9 7 7 0 6 9 6 9 6 9 6 9 6 9 6 9 6 9 6 9 7 7 0 6 9 6 9 6 9 6 9 6 9 6 9 6 9 6 9 6 9 6	BETT OCCOUNTIES OCCOUN

#### TABLE XIII—Continued

[Stiffener pitch=2.50 inches. Stiffener depth, D=0.500 inch]

Specific	Walaki	Area	Failing	Failing	Thic	kness			Num-			Stiffener	Percent	
Specifica- tion no.	Weight (pounds)	(square inch)	load	stress (lb./sq.in.)	Stiff- ener (inch)	Sheet (inch)	Length (inches)	Width (inches)	ber of stiff- eners	Area of sheet	Area of stiff- eners	width (W- inches)	rein- force- ment	Type of failure
EB-485b EB-486a EB-487a EB-487a EB-487a EB-4880a EB-4880a EB-4890b EB-4900a EB-4910b EB-4910a EB-4910b EB-4910b EB-4910b EB-4910b EB-4910b EB-4950b EB-4950b EB-4968 EB-4968 EB-4960b EB-4970a EB-4980b EB-4980b EB-4980b EB-4980b EB-5001a EB-5001a EB-5011a	2. 558 3. 823 3. 822 1. 306 1. 311 2. 622 6. 616 3. 939 1. 405 1. 438 2. 815 2. 821 4. 238 4. 238 4. 248 1. 467 2. 955 1. 495 1. 495 1. 495 1. 495 1. 491 1. 531 1. 531 1. 531 1. 531 1. 531 2. 622 2. 624 2. 626 2.	1. 0040 1. 0020 1. 0650 1. 0650 1. 0660 1. 0680 1. 0680 1. 0680 1. 0680 1. 0680 1. 0675 1. 1160 1. 1160 1. 1160 1. 1160 1. 11680 1. 1680 1. 1680 1. 1730 1. 1730 1. 1775 1. 2480 1. 2490 1. 2490 1. 2490 1. 2495 1. 2495	111, 400 93, 700 86, 000 119, 100 124, 800 -99, 100 95, 600 123, 000 114, 600 105, 800 92, 100 135, 500 121, 800 102, 800 102, 800 102, 800 102, 800 102, 900 1121, 800 102, 800 102, 800 1121, 800 102, 800 1121, 800 102, 800 1121, 800 102, 800 103, 500 1121, 800 104, 500 118, 100 114, 600 118, 100 114, 600 118, 100 114, 600 109, 500 117, 200 100, 700	111, 000 93, 500 85, 900 111, 600 117, 200 	. 050 . 050	.031 .030 .030 .031 .031 .031 .050 .053 .049 .049 .050 .050 .049 .049 .049 .049 .049 .049 .049 .04	8. 90 13. 34 13. 34 4. 30 8. 60 8. 59 12. 89 12. 89 12. 89 13. 87 13. 88 4. 60 9. 20 13. 83 13. 78 4. 48 8. 89 13. 34 8. 86 8. 99 12. 87 12. 87	9. 19 9. 27 9. 27 9. 49 9. 57 9. 50 8. 76 8. 78 8. 88 8. 89 8. 94 8. 99 9. 22 9. 22 9. 22 9. 49 9. 51	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	. 2865 . 2781 . 2781 . 2940 . 2965 . 2841 . 2945 . 2945 . 2945 . 2945 . 4385 . 4650 . 4290 . 4390 . 4490 . 4490 . 4490 . 4495 . 4350 . 4350	. 7175 . 7239 . 7239 . 7730 . 7685 . 7849 . 7705 . 6235 . 6225 . 6360 . 6280 . 6275 . 6750 . 6780 . 6775 . 6730 . 6745 . 7130 . 7230 . 7230 . 7210 . 7225 . 7690 . 7760 . 7760 . 7760 . 7760 . 7760 . 7760 . 7760 . 7760	1. 25 1. 25 1. 25 1. 50	69. 8 70. 7 70. 7 71. 4 71. 2 72. 2 71. 3 71. 2 55. 5 54. 0 58. 5 55. 8 55. 6 68. 0 57. 1 57. 4 57. 1 57. 8 57. 2 59. 5 59. 5 60. 6 60. 6 60. 8 60. 8	100011

TABLE XIV

#### [Stiffener pitch=2.50 inches]

Specifica- tion no.	Weight (pounds)	Area (square inch)	Failing load (pounds)	Failing stress (lb./sq. in.)	Thic Stiff- ener (inch)	Sheet (inch)	Length (inches)	Width (inches)	Number of stiff- eners	Area of sheet	Area of stiff- eners	Stiffener width and depth (W and D inches)	Percent rein- force- ment	Type of failure
EC-502a EC-502b EC-503b EC-504a EC-504a EC-504b EC-505a EC-506b EC-507b EC-507b EC-507b EC-508b EC-508b EC-509a EC-510a EC-511a EC-511a EC-512a EC-512b EC-512b EC-513b EC-514b EC-514b EC-516a EC-516a EC-516a EC-516a EC-516a EC-516b EC-516a EC-516b EC-517a EC-517a EC-518b EC-517a EC-518b EC-518b EC-518b EC-518b EC-518b EC-518b EC-518b EC-518b EC-518b EC-518b EC-519b EC-519b	0. 418     . 422     . 845     . 8482     1. 375     1. 266     1. 265     1. 886     1. 265     1. 743     1. 717     2. 586     1. 282     1. 290     2. 602     3. 890     3. 890     3. 890     3. 890     3. 900     1. 509     1. 509     1. 509     1. 609     1.	0. 2125 . 2140 . 2143 . 2233 . 2322 . 2135 . 2408 . 2400 . 2415 . 2400 . 2415 . 2685 . 2685 . 2685 . 2583 . 2583 . 3380 . 3395 . 3380 . 3395 . 3380 . 3416 . 4155 . 4160 . 4580 . 4580 . 5000 . 4545	11, 390 11, 970 10, 670 11, 600 10, 880 10, 850 10, 850 10, 10, 100 10, 300 10, 070 10, 550 9, 600 9, 550 9, 400 11, 300	53, 600 55, 900 49, 800 51, 950 44, 200 42, 100 42, 600 41, 900 33, 500 33, 500 33, 500 33, 500 33, 500 33, 500 33, 500 33, 500 33, 500 34, 600 34, 600 52, 500 57, 100 52, 700 47, 700 41, 600 41, 600 41, 600 41, 600 42, 400 43, 750 42, 600 44, 600 44, 600 44, 600 45, 750 42, 600 42, 400 48, 750 42, 600 24, 700 42, 600 24, 700	0.009 .009 .010 .010 .010 .010 .009 .009	0.009 010 010 010 010 010 010 010 010 010	6. 88 6. 90 13. 78 13. 79 9. 12 9. 18 18. 15 11. 34 12. 25 50 22. 69 22. 69 22. 69 22. 69 22. 69 22. 69 22. 69 13. 47 27. 02 35. 02 35. 02 35. 06 13. 47 27. 05 9. 18 18. 15 9. 18 19. 19. 19. 19. 19. 19. 19. 19. 19. 19.	8. 66 8. 68 8. 64 8. 71 8. 88 8. 65 8. 85 9. 12 9. 11 9. 17 9. 08 9. 19 9. 33 9. 33 9. 33 8. 66 8. 63 8. 89 9. 80 9. 80 90 90 90 90 90 90 90 90 90 90 90 90 90	444444444444444444444444444444444444444	0. 0780 - 0776 - 0868 - 0864 - 0871 - 0888 - 0885 - 0885 - 0885 - 0886 - 0820 - 0820 - 0820 - 0820 - 0825 - 0815 - 0835 -	0.1345 .1364 .1275 .1379 .1458 .1264 .1547 .1543 .1520 .1619 .1619 .1619 .1620 .1860 .1879 .1789 .2500 .2447 .2457 .2472 .2431 .1310 .1227 .1310 .1227 .1310 .1227 .1310 .1237 .1310 .1237 .1486 .1473 .1486	0. 75 . 75 . 75 . 75 . 75 . 75 . 1. 00 1. 00 1. 00 1. 00 1. 25 1. 25 1. 25 1. 25 1. 50 1.	59. 9 60. 2 56. 1 58. 0 59. 3 55. 8 60. 7 60. 7 60. 3 64. 3 67. 3 67. 4 67. 1 68. 1 73. 5 71. 0 71. 1 70. 7 71. 1 29. 2 33. 3 33. 2 33. 3 33. 5 38. 9 37. 4 42. 4 37. 4	HHAAAFHHGHFACFCCCCCCCCGGAAGEHHCCCCHHHJ

#### TABLE XIV-Continued

[Stiffener pitch=2.50 inches]

					Thic	kness						Stiffener width		
Specifica- tion no.	Weight (pounds)	Area (square inch)	Failing load (pounds)	Failing stress (lb./sq. in.)	Stiff- ener (inch)	Sheet (inch)	Length (inches)	Width (inches)	Number of stiff- eners	Area of sheet	Area of stiff- eners	and depth (W and D inches)	Percent rein- forced	Type of failure
20b	2. 965 4. 4200 1. 930 1. 930 1. 931 1. 930 1. 931 1. 930 1. 1112 2. 220 2. 3. 342 2. 149 2. 153 2. 1	0. 4560     . 4550     . 5030     . 5030     . 5030     . 5030     . 5050     . 5640     . 5590     . 5650     . 6350     . 6350     . 6350     . 6810     . 6810     . 6810     . 6850     . 68910     . 7000     . 4965     . 4965     . 4965     . 4960     . 4965     . 5930     . 5930     . 5930     . 5930     . 5930     . 5930     . 5930     . 5930     . 7650     . 7860     . 7860     . 6685     . 6750     . 6685     . 6750     . 6685     . 6750     . 6685     . 6750     . 6855     . 8590     . 8555     . 8590     . 8595     . 9855     . 9856     . 9850	15, 220 17, 300 17, 300 17, 300 17, 300 17, 300 17, 350 30, 110 33, 670 30, 110 22, 150 23, 150 24, 140 27, 150 28, 150 28, 150 29, 150 20, 15	33, 400 38, 900 34, 720 33, 450 34, 500 35, 600 35, 600 35, 600 35, 650 58, 700 32, 200 41, 100 32, 900 41, 900 41, 500 60, 900 60, 900 60, 900 78, 500 78, 500 78, 500 79, 600 96, 100 96, 100 96, 100 96, 100 96, 100 96, 100 96, 100 97, 400 98, 500 97, 100 98, 500	0. 009 0.009 0.009 0.009 0.009 0.009 0.009 0.009 0.009 0.009 0.011	0.030 0.230 0.229 0.229 0.229 0.229 0.230 0.230 0.330 0.330 0.349 0.449	22. 69 34. 02 34. 02 13. 50 13. 50 127. 04 40. 59 40. 59 13. 82 27. 50 11. 33 12. 68 13. 80 27. 50 11. 33 12. 68 13. 80 27. 50 11. 33 12. 68 13. 80 13. 80 13. 80 13. 80 14. 80 15. 80 16. 80 16. 80 16. 80 16. 80 17. 80 18. 80 1	9.9.9.4383757227659917726599188889995981866666559899999999999999	444444444444444444444444444444444444444	0. 2739     . 2655     . 2635     . 2635     . 2635     . 2635     . 2735     . 2735     . 2735     . 2735     . 2735     . 2735     . 2735     . 2735     . 2735     . 2735     . 2735     . 2735     . 2736     . 2360	0. 1821 .1890 .1895 .2365 .2310 .2273 .2283 .2285 .1265 .1290 .1280 .1330 .1415 .1715 .18160 .1745 .18160 .1745 .1825 .1820 .2240 .2210 .2210 .2210 .2210 .2210 .2210 .2100 .2180 .2190 .4116 .4180 .4110 .4193 .4180 .4193 .5018 .5018 .5038 .5118 .5038 .5118 .5038 .5118 .5038 .5118 .5038 .5118 .5038 .5118 .5038 .5118 .5038 .5118 .5038 .5118 .5038 .5118 .5038 .5038 .5118 .5038 .5038 .5118 .5038 .5	1.25 1.25 1.25 1.50 1.50 1.50 1.50 1.50 1.50 1.50 1.5	37. 8 4 4 3 5 0 6 1 2 2 2 4 5 3 3 9 9 9 9 4 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	СНИССССССОНИ▲НОСИНИНОСССССОВИНССООВОВООООООООООВОВНИТОТЬОВОООООВОВИТИВОВТИНЕТЬ В СОСТОВИНИТЬ В СОСТОВИТЬ В СОСТОВИНИТЬ В СОСТОВИТЬ В СОСТОВИНИТЬ В СОСТОВИТЬ В СОСТОВИНИТЬ В СОСТОВИТЬ В СОСТОВИТЬ В СОСТОВИНИТЬ В СОСТОВИТЬ В

#### TABLE XIV-Continued

[Stiffener pitch=2.50 inches]

					Thic	kness						Stiffener width	l.	
Specifica- tion no.	Weight (pounds)	Area (square inch)	Failing load (pounds)	Failing stress (lb./sq. in.)	Stiff- ener (inch)	Sheet (inch)	Length (inches)	Width (inches)	Number of stiff- eners	Area of sheet	Area of stiff- eners	and depth (W and D inches)	Percent rein- force- ment	Type of failure
ECC-5656ab  ECC-5656ab  ECC-5656ab  ECC-5656ab  ECC-5656ab  ECC-5656ab  ECC-5656ab  ECC-5656ab  ECC-56578ab  ECC-56578ab  ECC-5678ab  ECC-5678ab  ECC-5778ab  ECC-	4. 947 4. 947 4. 947 4. 947 4. 947 4. 947 4. 947 4. 947 4. 947 4. 947 4. 947 4. 947 4. 947 4. 947 4. 947 4. 947 4. 947 4. 948 4. 949 4. 488 4. 948 4. 849 4. 849 4. 849 4. 849 4. 849 4. 849 4. 849 4. 849 4. 849 4. 849 4. 849 4. 849 4. 849 4. 849 4. 849 4. 873 4. 849 4.	0. 9465	102, 550 106, 300 187, 400 187, 500 187, 400 188, 450 189, 800 189, 900 102, 900 102, 900 103, 900 118, 700 118, 700 118, 700 118, 750 113, 750 113, 750 114, 400 115, 600 117, 700 118, 900 118, 800 118, 900 118, 900 118, 900 118, 900 118, 800 118	108, 300 112, 100 91, 900 93, 100 94, 900 92, 200 86, 400 88, 500 88, 200 88, 200 124, 300 145, 600 124, 300 145, 600 127, 200 139, 700 140, 300 120, 800 75, 200 121, 300 120, 800 75, 200 121, 300 120, 800 77, 200 121, 300 121, 300 121, 300 122, 200 123, 300 124, 300 125, 200 126, 200 127, 300 128, 300 129, 200 120, 800 121, 300 121, 300 121, 300 122, 200 123, 300 124, 300 125, 200 127, 300 128, 300 129, 300 120, 800 121, 300 121, 300 122, 200 123, 300 124, 300 125, 700 127, 300 128, 300 129, 300 120, 800 121, 300 121, 300 122, 200 123, 300 124, 300 125, 700 127, 300 128, 700 129, 900 120, 800 121, 300 121, 300 122, 200 123, 300 124, 300 125, 700 127, 300 128, 500 129, 500 120, 800 121, 300 121, 300 122, 200 123, 300 124, 300 125, 700 127, 300 128, 500 129, 500 120, 800 121, 300 121, 300 122, 200 123, 300 124, 300 125, 700 127, 300 128, 500 129, 500 120, 800 121, 300 122, 200 123, 300 124, 300 125, 500 127, 300 128, 500 129, 500 120, 800 121, 300 122, 200 123, 300 124, 300 125, 500 126, 500 127, 300 128, 500 129, 500 120, 800 121, 300 122, 800 123, 800 124, 800 125, 800 127, 800 128, 800 129, 800 120, 800 120, 800 121, 800 121, 800 122, 800 123, 800 124, 800 125, 800 126, 800 127, 800 128, 800 129, 800 120, 80	0. 030	0.050 .050 .050 .050 .050 .050 .050 .05	18. 432 27. 49 27. 49 27. 49 27. 49 27. 49 21. 31 22. 70 23. 40 23. 40 24. 40 25. 59 20. 64 27. 59 20. 64 27. 59 20. 64 27. 59 28. 69 29. 16 20. 64 20. 6	967 979 914 14 14 19 20 46 24 20 50 44 76 78 22 78 78 78 78 78 78 20 78	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	0.4480 -4480 -4480 -4480 -4480 -4570 -4570 -4580 -4570 -4580 -4580 -4610 -4650 -4650 -4650 -6850 -6851 -6850 -6851 -6850 -6852 -6850 -6851 -6850 -6852 -6852 -6852 -6852 -6852 -6853 -6852 -6853 -6852 -6853 -6852 -6853 -6852 -6853 -6852 -6853 -6852 -6853	0. 4985	1.00 1.00 1.02 1.02 1.03 1.03 1.03 1.03 1.03 1.03 1.03 1.03	90.27.96.07.20.07.20.07.20.07.20.07.20.07.20.07.20.00.07.20.07.20.00.00.00.00.00.00.00.00.00.00.00.00.	1400-1

)

TABLE XV

Specifi-	Weight	Area	Failing	Failing	Thiel	rness	Length	Width	Number	Area	Area of stiff-	Stiff- ener depth	Percent rein-	Type of
cation no.	(inches)	(square inch)	load (pounds)	stress (lb./sq. in.)	Stiffener (inch)	Sheet (inch)	(inches)	(inches)	of stiff- eners	of sheet	eners	(D in- ches)	force- ment	failure
F-609a F-609b F-610a F-611a F-611a F-612b F-612a F-613b F-613b F-614b F-615a F-615a F-616a F-616a F-617a F-617a F-617a F-618a F-618b F-618a F-618a F-618a F-618a F-618a F-618a F-618a F-618a F-618a F-618a F-618a F-618a F-618a	0. 590 . 594 1. 352 1. 360 2. 358 2. 232 1. 010 1. 000 2. 228 3. 541 3. 583 1. 467 1. 446 4. 816 4. 816 4. 811 1. 591 1. 591 1. 591 1. 581 3. 557 5. 557	0. 2275 2280 2620 2620 3160 2998 3900 3860 4330 4740 4825 5680 5595 6203 6160 6465 6105 6090 6885 7450	14, 260 14, 325 16, 200 15, 400 16, 300 13, 600 24, 150 24, 770 25, 020 38, 480 38, 610 24, 770 29, 150 24, 770 29, 150 40, 700 41, 150 41, 300 42, 150 41, 300 42, 150 41, 300 42, 700	62, 700 63, 000 61, 750 58, 700 51, 600 45, 400 57, 200 57, 200 41, 200 67, 800 69, 000 40, 250 40, 250 60, 650 61, 000 61, 00	0.010 010 009 009 009 010 010 009 009 009	0, 010 010 010 010 010 010 010 030 03	9. 06 9. 10 18. 05 18. 15 26. 04 26. 01 9. 06 9. 06 18. 02 26. 08 25. 99 9. 04 9. 04 9. 04 9. 06 9. 07 17. 98 18. 04 26. 03 26. 04 9. 07	8. 50 8. 59 8. 59 8. 49 8. 49 8. 44 8. 50 8. 44 8. 53 8. 43 8. 53 8. 44 8. 53 8. 44 8. 53 8. 44 8. 54 8. 54	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	0. 0850 0. 0847 0. 0859 0. 0859 0. 0849 0. 0849 2. 2541 2. 2652 2. 2618 2. 2645 4. 150 4. 150 4. 305 4. 4150 4. 415	0.1425 .1433 .1761 .1762 .2311 .2149 .1359 .1310 .1702 .2112 .2180 .1530 .1445 .2023 .2180 .2160 .2160 .2161 .2041 .2051	0. 50 . 50 1. 00 1. 50 1. 50 . 50 1. 00 1. 50 1.	58. 7 58. 9 63. 8 69. 8 68. 2 30. 4 35. 6 35. 5 40. 5 40. 5 30. 2 22. 8 22. 8 22. 8 22. 3 30. 2 29. 2 30. 3 30. 2 30. 2	GGGJIIAAIIIIHHIIIIHHIII

Table XVI—Corrugated stainless steel specimens

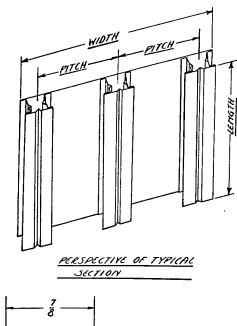
Cnooifi		Area	Failing	Failing	Thiel	kness	·	W71 34h	Number	Area of	Area corru-	Pitch		
Specifi- cation no.	Weight (pounds)	(square inch)	heaf	stress (lb./sq. in.)	Stiffener (inch)	Sheet (inch)	Length (inches)	Width (inches)	of corru- gations	sheet (square inch)	gated (square inch)	(inches)	L/p	R/t
I-la	0, 144	0. 1215	5,000	41, 150	0.010		4.0	8. 563	8		0. 1215	1,00	22. 8 22. 8	25 25
I-1b	139	. 1258	7,700	61, 200	. 010		4.0	8. 563	8 8		. 1258	1.00 1.00	22.8 45.6	25
I-2a I-2b	.139 .268 .268	. 1170	9, 350	79, 950	.010		8. 0 8. 0	8. 469 8. 469	8		. 1170 . 1170	1.00	45.6	25 25
I-2b I-3a	.268	.1170	7,900 10,400	87,500	.010		12.0	8.50	8		.1165	1,00 1,00 1,00	45. 6 68. 4	25
1-38 I-3b	.400 .410	.1165 .1195	11,000	92,000	.010		12.0	8.50	8		1195	1.00	68. 4 22. 35	25 40. 5
I-4a	.120	. 1048	9, 330	89,000	.010		- 4.0 4.0	9.50	6		. 1048	1.50 1.50	22.35	40.5
I–4b	. 125	. 1092	4 500	41, 200	.010		4.0 8.0	9.50 9.625	6		. 1092 . 1092	1.50	22. 35 44. 7	40.5
I-5a	. 250	. 1092	6,000	55,000	010		8.0	9. 625	6		1092	1.50 1.50	44.7	40.5
I-5b I-6a	. 250	. 1092 . 1131	6,000 6,250 6,400 7,800 4,330	56,600	.010		12.0	9, 594	6		. 1131	1. 50 1. 50 2. 00 2. 00 2. 00 2. 00 2. 00	67. 1	40.5
I-6b	374	. 1090	7, 800	71, 500	.010		12.0	9, 594	6		. 1090	1.50	67.1	40. 5 66
I–7a	. 100	. 0875	4,330	49, 500	.010		4.0	8. 50 8. 50	4 4		. 0875	2,00	22, 19 22, 19 44, 38 44, 38 66, 55 66, 55 22, 8 22, 8 45, 6	66
I-7b	.100	. 0875	3, 750 4, 450	42,900	010		8.0	8. 625	1 1		.0874	2.00	44. 38	66
I-8a I-8b	. 200 200	. 0874	3 900	44,600	.010		8.0	8, 625				2.00	44.38	66
T0a	300	.0875	3, 900 3, 500	40,000	.010		12.0	8. 594	4		. 0875	2.00	66.55	66 66
I-9b	.300	.0875	3,600	41, 100	. 010	<u>-</u>	12.0	8. 594 8. 469	8		. 0875 . 2430	2.00	22.8	12.5
I–10a I–10b	. 278	. 2430	3, 600 36, 300 33, 650 37, 400 27, 100	149,300	.020	<del>-</del>	4.0 4.0	8. 469	8		. 2335	2.00 1.00 1.00 1.00	22.8	12. 5 12. 5
I-10b I-11a	. 267 . 550	. 2335	33,650	155 400	. 020		8.0	8. 469	8		. 2335 . 2405	1.00	45.6	12.5
I-11b	- 550	. 2405	27, 100	112,700	. 020		8.0	8.469	8		. 2405	1.00	45. 6 68. 4	12.5 12.5
I–12a I–12b	. 863	. 2515	28, 000 33, 400	111, 300	. 020		12.0	8. 531	, ,	l	. 2515 . 2405	1.00	88.4	12.5
I-12b	.826 .255	. 2405	33, 400	138,800	.020		12.0	8. 531 9. 531	8		2230	1.50	68. 4 22. 35 22. 35	20.3
I–13a I–13b	255	. 2230	29, 900 30, 300 26, 200	134,000	1 020		4.0 4.0	9, 531	6		2230	1.50 1.50	22.35	20.3
I-13D I-148	. 255 . 533 . 538	.2350	26, 200	111, 300	. 020		8.0	9. 469 9. 469	6		. 2350	1.50	44.7	20.3 20.3
I-14b	.538	. 2350	29, 650 28, 400	126, 100	.020		8.0	9.469	6		. 2350 . 2290	1.50	44.7 67.1	20.3
I-15a I-15b	.785 .795	. 2290	28, 400	124,000	.020		12. 0 12. 0	9. 531 9. 531	6		. 2290	1.50	67. 1	20.3
1-15b	.795	. 2315 . 1860	28, 200 18, 500 18, 500 19, 450 19, 800	99 500	020		4.0	8. 469	4		1860	1. 50 2. 00 2. 00 2. 00 2. 00 2. 00	67. 1 22. 19 22. 19	33
I–16a I–16b	.213 .213	. 1860	18, 500	99, 500	.020		4.0	8.469	4		. 1860	2.00	22.19	33
I-17a	.435	.1900	19,450	102, 300	.020 .020		8.0	8.469	4		. 1900	2.00	44. 38 44. 38	33
I-17b	435	. 1900	19,800	104, 200	.020		8. 0 12. 0	8. 469 8. 594	4		. 1923	2.00	66. 55	33
I-18a I-18b	.660	. 1923 . 1923		100,900	.020		12.0	8. 594	4		1923	2.00	66, 55	33
I-19a	. 660 . 435	. 1923	65, 600	41, 150 61, 200 67, 950 67, 500 92, 000 88, 000 41, 200 55, 000 57, 250 49, 500 42, 900 41, 100 40, 000 41, 100 112, 700 1113, 300 126, 100 121, 70	.030		4.0	8, 531	8		. 3800	2.00 1.00 1.00	22.8	8.3
I-19a I-19b I-20a I-20b	. 435 . 855	.3800	18, 400 65, 600 67, 700 69, 700 24, 000 50, 100	196,000	.030		4.0	8. 531	8		.3800	1.00	22.8 45.6	20. 3 33 33 33 33 33 8. 3 8. 3 8. 3 8. 3
I-20a	- 855	. 3735	69,700	160, 000 64, 300 125, 800 117, 800 167, 000	.030		8.0 8.0	8. 50 8. 50	8		3835	1.00 1.00	45.6	8.3
I-20b I-21a	. 877	. 3835 . 3980	24,000	125 900	.030		12.0	8, 531	1 8		3980	1.00	68.4	8.3
I-218 I-21b	1.366 1.362	.3980	46, 800	117, 800	.030		12. 0 12. 0	8, 531	8		. 3970	1.00	68.4	8.3 13.5
I-22a	. 384	.3440	46, 800 57, 500 52, 050	167,000	.030		4. 0 4. 0	9. 50	6		. 3440	1.50 1.50 1.50	22.35 22.35	13.5
I-22a I-22b	. 384	. 3440	52,050	151, 200 154, 600	.030		4. 0 8. 0	9. 50 9. 50	6			1.50	44.7	13.5
I-23a 1 I-23b	. 795	. 3475	53,700	154,600	030		8.0	9.50	6		3310	1.50	44.7	13.5
I-23D I-248	.758 1.168	.3310	53, 700 18, 600 42, 600 42, 750 45, 500	56, 200 79, 800 78, 100 152, 100 160, 000 141, 500	.030 .030 .030		12.0	9.50	6		3400	1.50 1.50 1.50 2.00 2.00	67.1	13.5
I-24b	1. 145	.3340	42,750	78, 100	.030		12.0	9. 50	6	I	. 3340	1.50	67. 1 22. 19	13.5
T-25a	.341	. 2980	45, 500	152, 100	.030		4.0 4.0	8. 50 8. 50	4 4		. 2980 . 3162	2.00	22, 19	22 22 22 22 22
I-25b	.362	.3162	50,600	160,000	.030		8.0	8, 50	.4		. 2930	2 (8)	44.38	22
I–26a I–26b	. 670 . 670	. 2930 . 2930	41, 450 41, 900		.030		8.0	8.50	4		. 2930	2.00	44.38	22
I-27a	.950	2770	32, 200	116, 300 89, 750	.030		12.0	8.50	4		. 2770	2.00 2.00	66. 55 66. 55	22 22
Î-27b	.950	2770	24, 850	89, 750	. 030		12.0	8.50	4		. 2770	2.00	00.00	عما

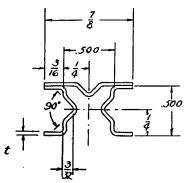
<sup>&</sup>lt;sup>1</sup> Tested on knife-edges.

31

Table XVI—Corrugated stainless steel specimens—Continued

Specifi- Wolght		Area	Failing	Failing	Thickness		Length	Width	Number	Area of sheet	Area corru-	Pitch	7/0	R/t
cation no.	Weight (pounds)	(square inch)	load (pounds)	stress (lb./sq. in.)	Stiffener (inch)	Sheet (inch)	(inches)	(inches)	of corrugations	(square inch)	gated (square inch)	(inches)	L/p	10/1
I-28a	.329	. 2881	40, 570	140, 700	. 019	0.005	3. 99	8. 47	8	0.0423	. 2458	1.00	21.4	
I-28b	. 340	. 2987	43,680	146, 400	.019	. 005	3.98 8.0	8. 54 8. 50	8 8	.0426	. 2561 . 2490	1.00 1.00	21. 4 43. 0	
I-29a I-29b	. 667 . 663	. 2915	36, 200 40, 500	124, 200 139, 700	· 019	.005	7.99	8. 52	8	.0426	. 2474	1.00	42.9	
I-30a	1 007	.3000	43.075	110 250	.019	. 005	11.97	8.48	8	.0424	.2576	1.00	69.8	
I-30b	1.009	. 2940	32, 050 43, 210	109,000	.020 .019	. 005 . 010	11. 99 3. 97	8. 52 8. 52	8 8 8	. 0426 . 0852	. 2514 . 2423	1.00 1.00	69. 9 21. 0	
I-31a I-31b	.372	. 3275 . 3328	46,930	132,000 140,900	020	.010	3.98	8.50	8	.0850	2478	1.00	21.0	
I-32a	.379	, 3243	42, 330	130, 400	.019	. 010	7.98	8. 58	8	.0858	. 2385	1.00	42.1 42.2	
I-32b	.752	. 3290	43, 190	131, 200	.019	.010	7.99	8. 58 8. 55	************	. 0858 . 0855	. 2432 . 2517	1.00 1.00	63.1	
I-33a I-33b	1. 152 1. 126	.3372 .3288	33, 990 34, 410	100, 900 104, 600	.019	.010	11.97	8.48	8	. 0848	. 2440	1.00 1.00	63. 3	
I-34a	.415	. 3643	46,990	129,000	.019	. 014	3.98	8. 52	8	. 1193	. 2450	1.00	21. 2	
I-34b	. 420	.3690	44, 560	120, 900	.019	. 015	3.98 8.0	8. 49 8. 48	8	. 1272 . 1187	. 2418 . 2439	1.00 1.00	21. 2 42. 5	
I-35a I-35b	.830 .838	.3626 .3670	42, 610 43, 180	117,500 117,600	.019	.014	7.98	8.50	8	. 1275	. 2395	1.00 1.09 1.00	42, 4	
I-36a	1.285	.3746	34, 590	117, 600 92, 300 98, 000	.019	. 015	11.99	8. 50	8	. 1275	. 2471	1.00	63.6	
I-36b	1. 278	. 3730	36, 560	98,000	.019	. 014	11.98 4.40	8.52 8.50	8 8	.1192	. 2538 . 2415	1.00	63. 6 21. 4	
I-37a I-37b	. 471 . 458	, 4115 , 4000	43, 400 43, 900	105, 200	.020	.020	4.0	8.50	8	.1700	. 2300	1.00	21.4	
I-38a	928	4050	46, 980	105, 200 109, 800 116, 000	.019	. 019	7.99	8, 51	8	. 1618	. 2432	1.00	42.7	
I-38b	.916	. 4000	40, 200	100,500	. 020	.020	8.0	8. 50 8. 50	8 8	. 1700 . 1615	. 2300 . 2506	1.00	42.7 64.0	
I-39a	1. 412 1. 406	. 4121 . 4104	32, 920. 38, 480	79, 800 93, 700	.019	.019	11.97 11.98	8.49	8	1613	. 2491	1.00	64.0	
I-39b I-40a	.571	5013	59, 230	i 118.300	.019	.029	3.98	8.55	8	. 2480	2533	1.00	21.8	
I-40b	. 564	. 4930	59, 450	120, 500 117, 000	.019	. 029	4.0	8.49	8	. 2460	. 2470 . 2473	1.00	21.9 43.7	
I-41a	1. 130 1. 100	. 4947	57, 790	117,000	.019	.029 .029	7.98 7.98	8. 53 8. 49	8	. 2474 . 2460	2355	1.00	43.7	
I-41b I-42a	1.100	. 4815 . 4888	56, 250 43, 200	88.400	.019	.029	11.98	8.50	8	. 2465	. 2423	1.00 1.00	65.6	
I-42b	1.652	4823	44, 050	116, 900 88, 400 91, 300	. 019	.028	11.97	8.49	8	. 2460	. 2363	1.00 2.00	65.6 21.1	
I-43a	1. 652 . 267	. 233	26,000	1 113.600	. 020	.005	4.0	8.50 8.50	4	.0425	1905	2.00	21.1	
I-43b I-44a	. 267	. 233	25, 200 26, 250	108, 200 110, 400	.020	.005	7.98	8.58	4	0429	. 1951	2.00	42.1	
I-44b	. 533	2335	24,800	106,300	.020	,005	7.98	8,58	4	. 0429	.1906	2.00	42.1 63.2	
I-45a	. 533 . 818	. 2385	23, 600	98, 900	.020	.005	11. 98 12. 0	8.58 8.57	4 4	.0429	. 1956 . 1936	200	63.2	
I-45b I-46a	.812	. 2365 . 264	21, 900 21, 500	92, 600 81, 500 92, 400	.020	.005 .010	3.97	8.50	4	. 0850	.1790	2.00	63. 2 20. 8	
I-46b	.300 .302	265	I 24 500	92, 400	.020	.010	3.98	8. 52	4	.0852	. 1798	2.00	20.9 41.8	
I-47a	. 625	. 274 274	25, 400	1 92,700	.020	.010	7. 98 7. 97	8. 55 8. 55	4	.0855 .0855	. 1885 . 1885	2.00	41.8	
I-47b I-48a	. 625 . 945	. 274 . 2755	25, 400 27, 200 27, 500	99,300	020 020	.010	12.0	8.61	4	.0861	. 1894	2,00 2,00 2,00	62.9	
I-48b	. 945	. 2750	26,500	99, 900 96, 200 90, 800	.020	. 010	12.0	8, 54	4	.0854	. 1896	2.00	62.9	
I-49a	. 367 . 366	. 322	29, 220 31, 020	90,800	.019	.015	3, 98 3, 97	8.52 8.52	4	.1278 .1278	1942	2.00	21. 2	
I-49b I-50a	. 366	.322	27, 760	96, 300 87, 900 86, 300	020	.015	7.99	8. 52 8. 49	4	. 1273	. 1881	2,00	42.5	
I-50a	. 715 1. 073	. 3170	27, 360	86,300	.019	.015	7, 97	8.50	1 4	. 1274	. 1896 . 1852	2.00	42.4 63.8	
I-51a	1. 073	. 3130	20,770	66,400	.020	.015 .015	11.99 11.98	8.52 8.64	4	. 1278 . 1295	1843	2.00	63.8	
I-51b I-52a	1. 075	.3138	21, 150 29, 440	67, 400 84, 700	.020	019	3.94	8, 64 8, 53	4	. 1279	. 2199	2.00	21.2	
I-52b	. 397	. 3455	30, 190	87, 400	.020	.019	3.96	8.53	4	. 1279	.2176	2.00	21. 3 43. 1	
I-53a	. 792	. 346	23, 650 27, 000	68, 400	.020	.019	8. 0 7. 98	8. 52 8. 51	4	.1278 .1277	. 2182 . 2223	2.00	43.0	
I-53b I-54a	1. 182	350	27,000	77, 200 66, 700 63, 000 78, 600	.019	.019	11.96	8.46	4	. 1610	. 1845	2.00	64.4	
I-54b	1, 202	. 3511	23, 030 22, 110	63,000	. 019	. 019	11,96	8. 51	1 1	. 1617 . 2570	. 1894 . 1730	2.00	64. 4 22. 4	
I-55a	. 491	. 4300 . 4345	33, 780	78, 600 73, 800	.019	.030	3. 99 3. 98	8. 56 8. 51	4	2465	1880	2,00	22. 4 22. 4	
I-55b I-56a	. 495	4345	32, 040 31, 020	71, 700	. 019	.029	7.99	8. 57	4	. 2485	. 1845	2.00	44.9	
I-56b	. 993	. 4376	37, 460	85, 700	.019	. 029	8.0	8. 50 8. 52	1 4	. 2460 . 2470	. 1916 . 1865	2.00	44.9 67.4	
I-57a	1. 486 1. 486	. 4335 . 4330	22, 980 25, 680	53, 000 59, 300	.019	.029	11.99 12.0	8. 52	1 4	2470	1860	2.00	67. 5	
I-57b	1. 450	1 . 3030	20,000	1 58,500	1 .019	.028	1	1	1 -	1	1	1	1	1





STIFFENER SECTION
A.6 C SERIES
FIG.-1

SHEET THICKNESS

SHEET THICKNESS

SHEET THICKNESS

SHEET THICKNESS

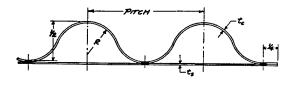
F SECTION

STIRRENER THICKNESS

F SECTION

FAG. 2

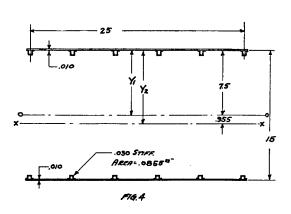
## CORRUGATED STAINLESS STEEL I- SERIES

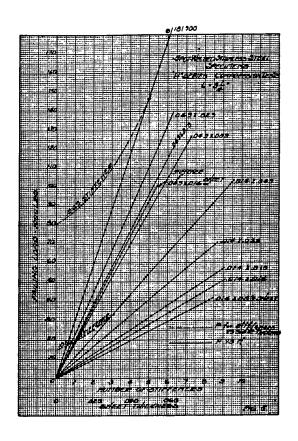


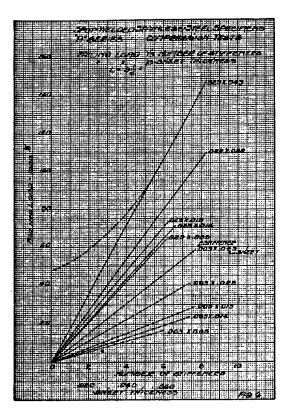
PITCH IN.	R IN.	RADIUS OF GYRATION S
1.00	.250	.1754
1.50	.405	./78 <b>9</b>
2.00	.660	./803

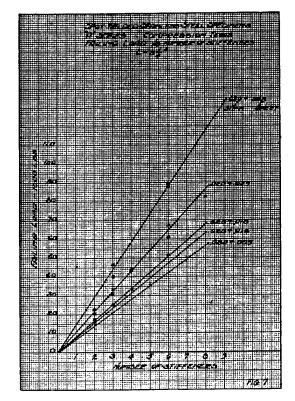
F14.3

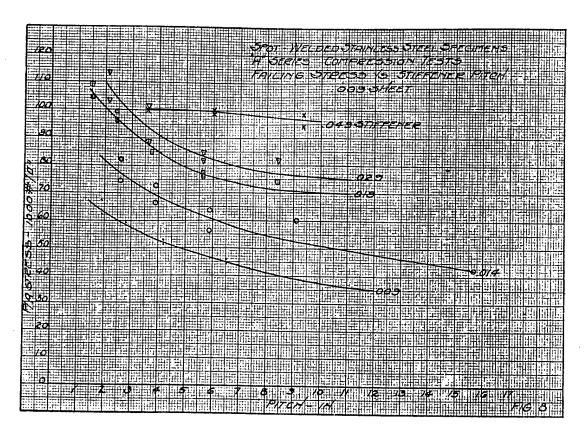
## SIMPLE BOX BEAM

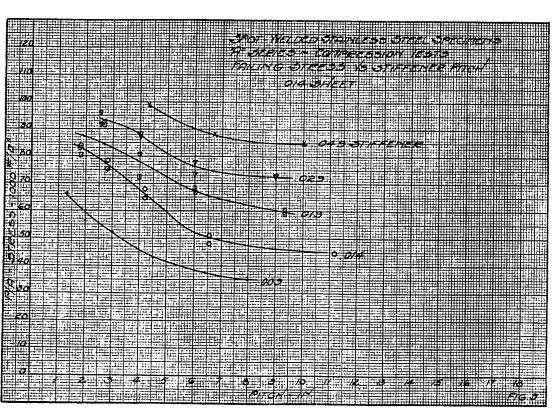


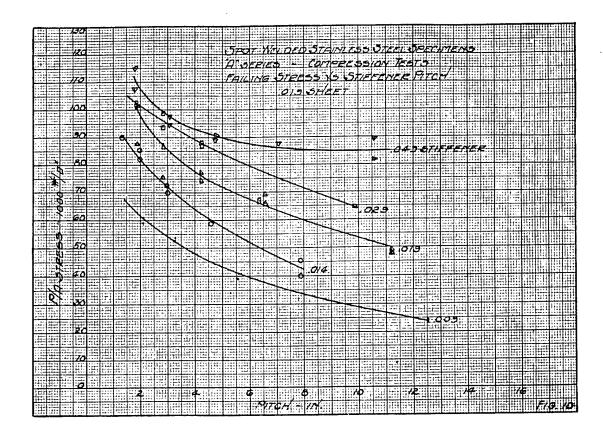


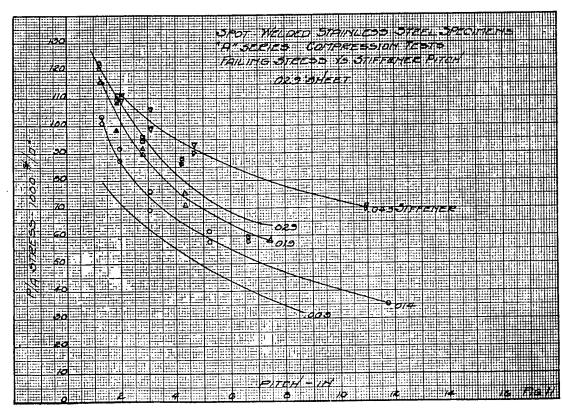


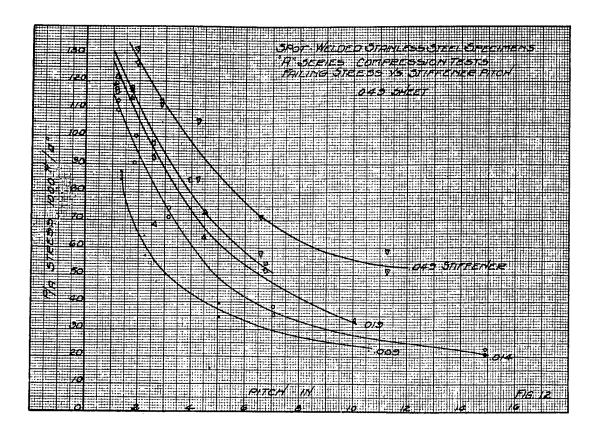


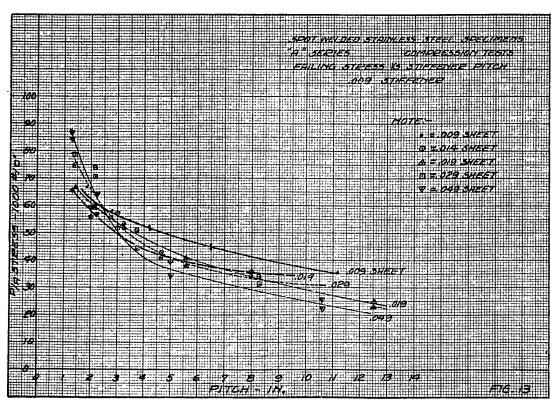


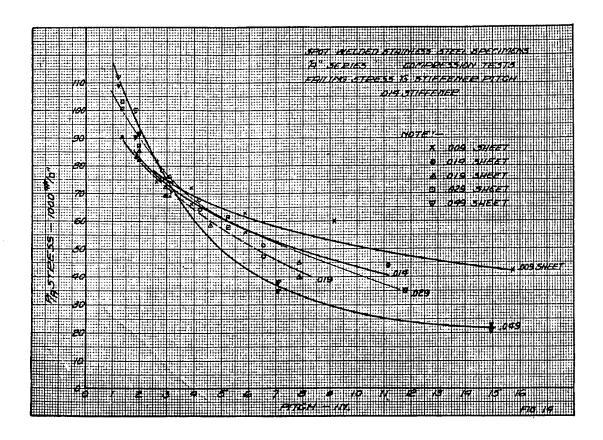


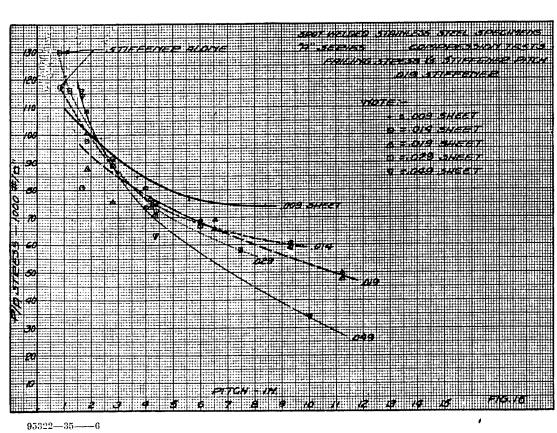


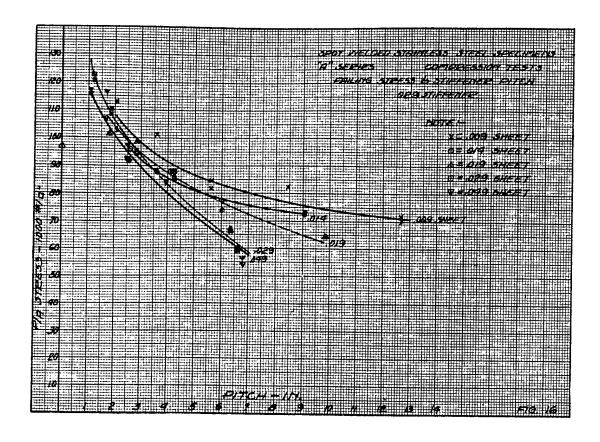


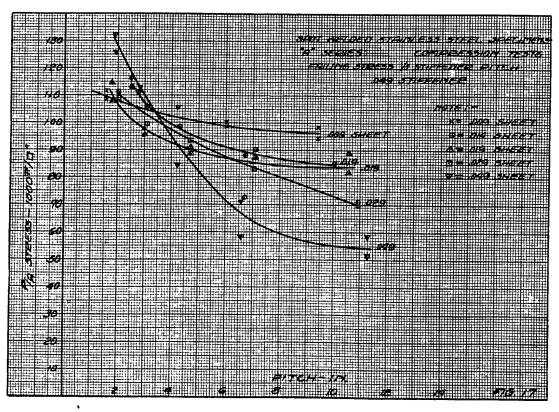


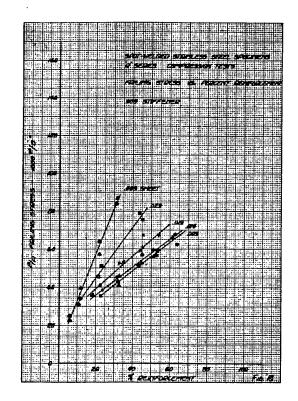


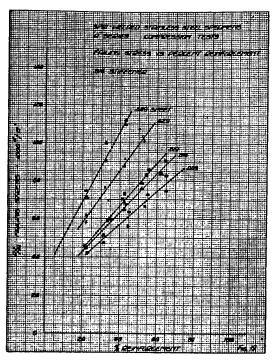


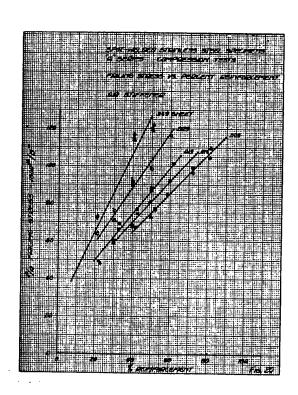


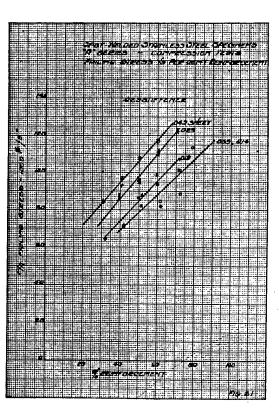


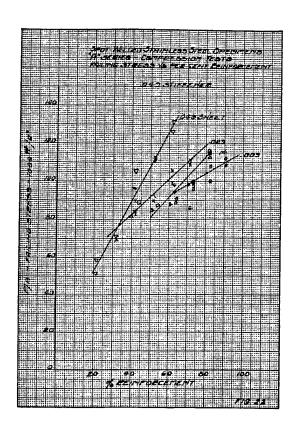


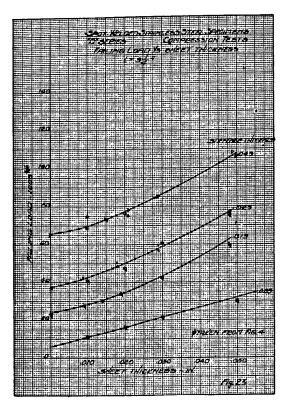


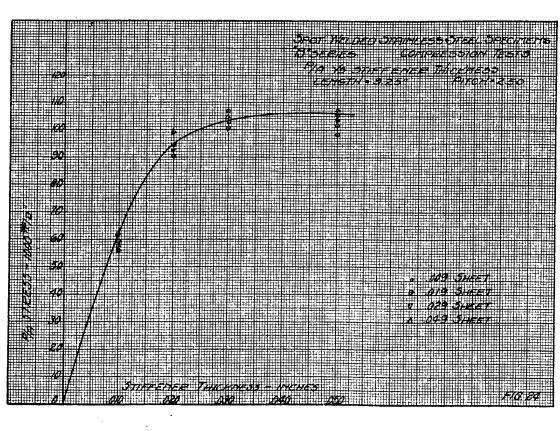




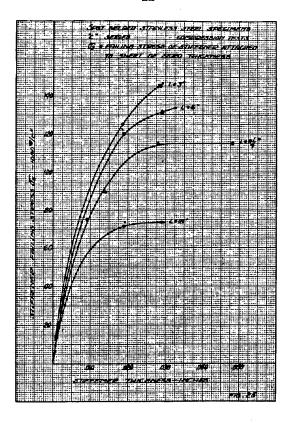


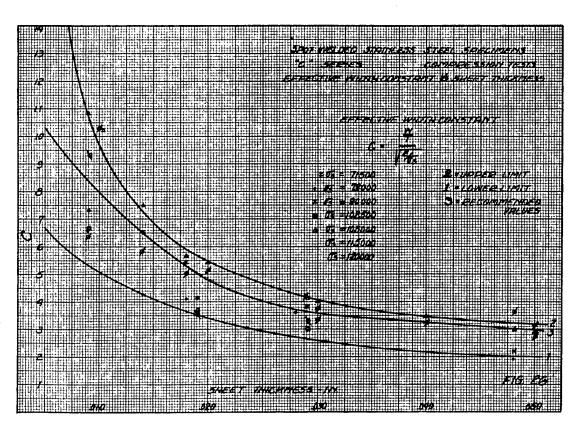


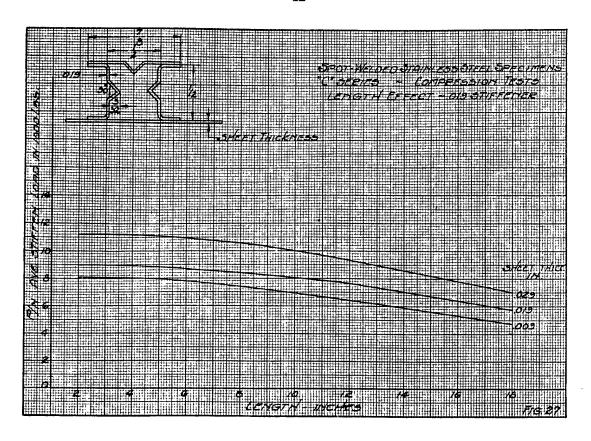


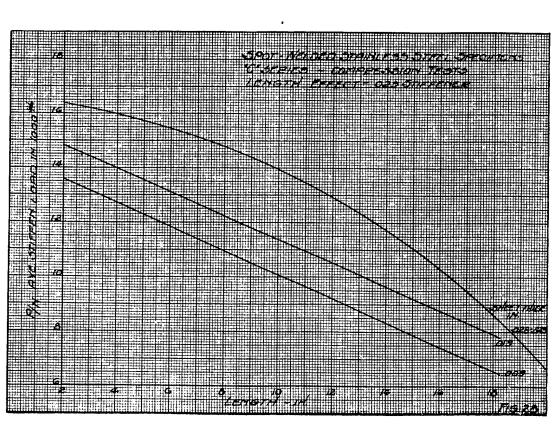


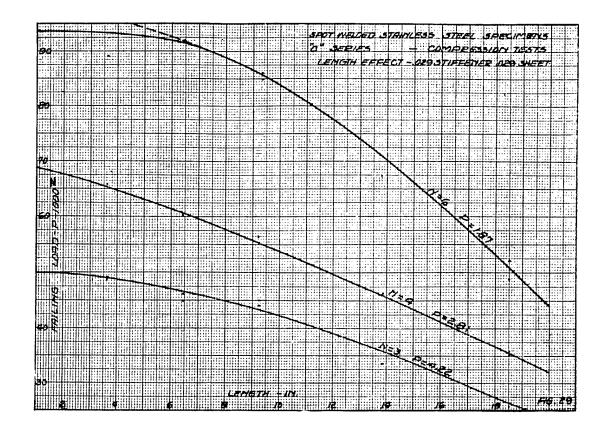
, J.

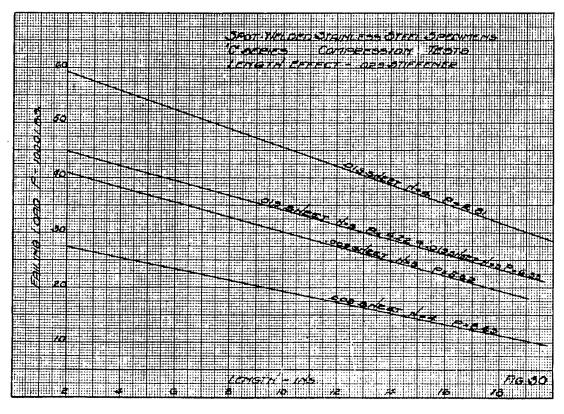


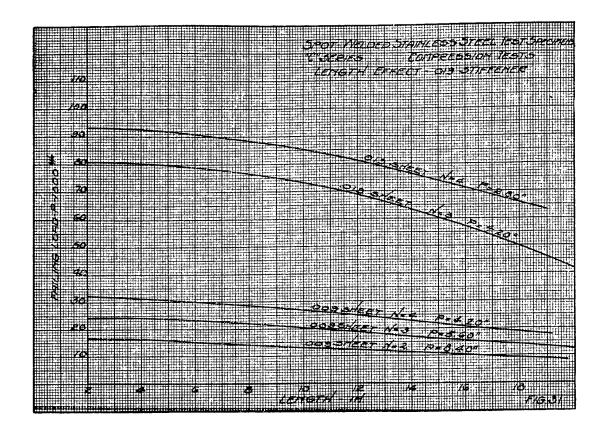


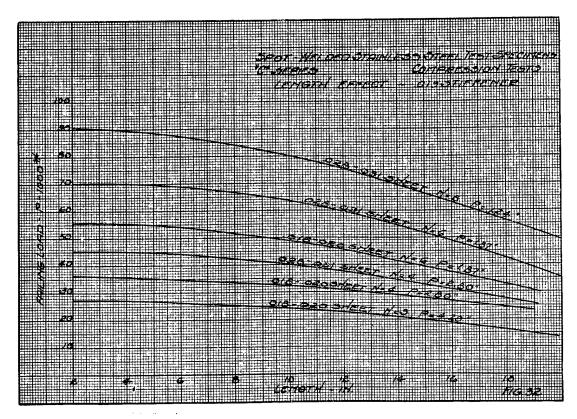


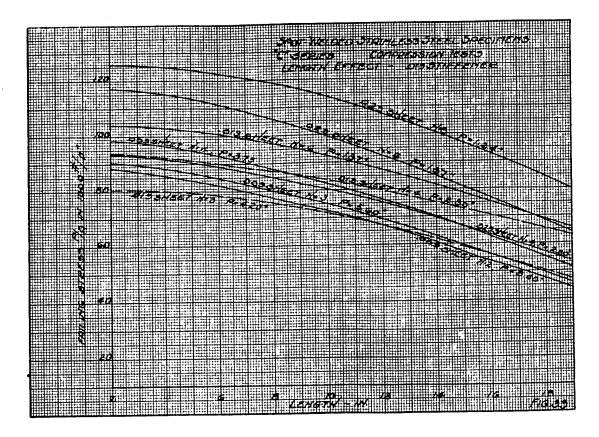


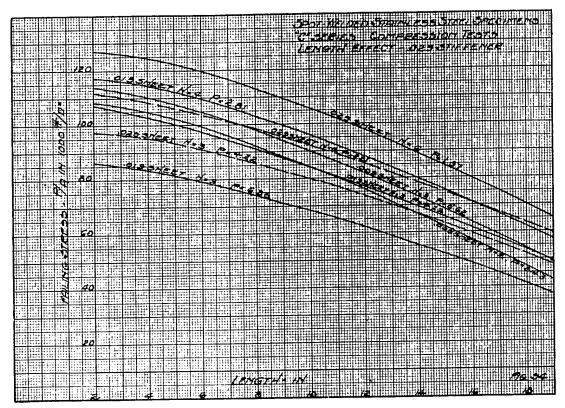


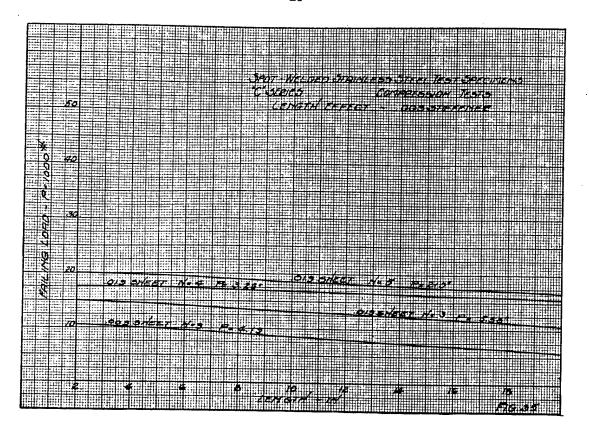


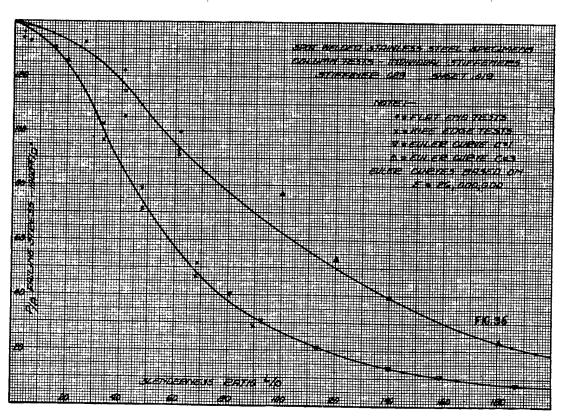


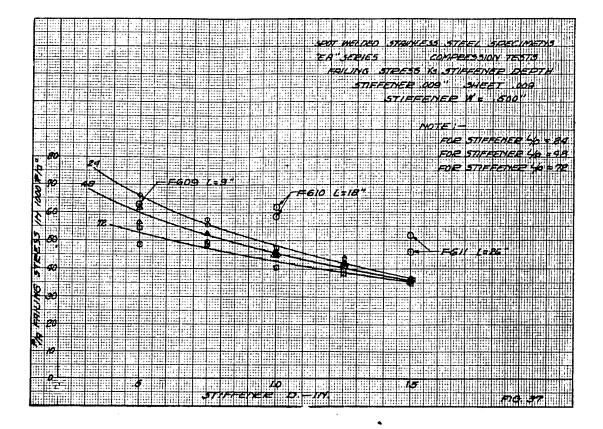


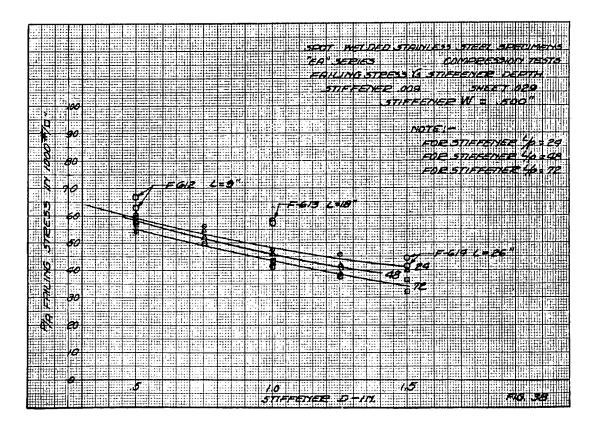




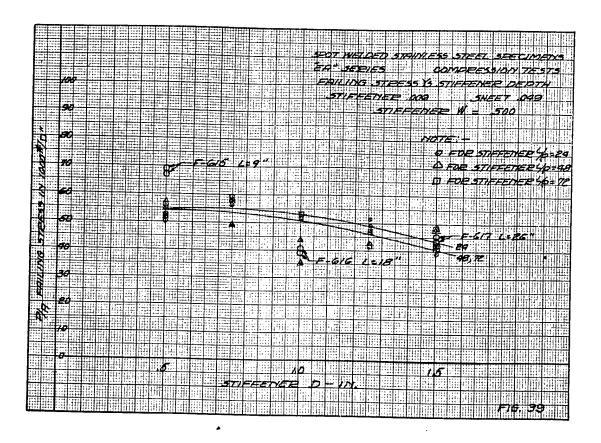


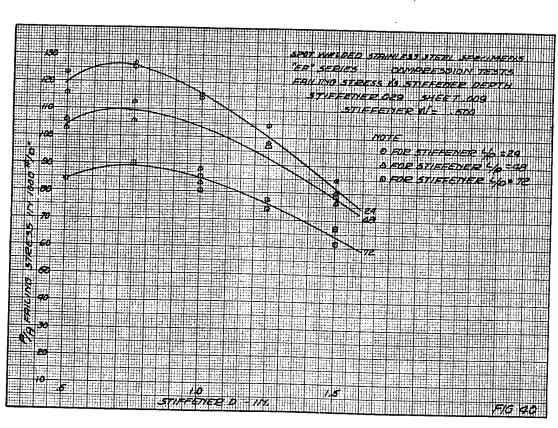


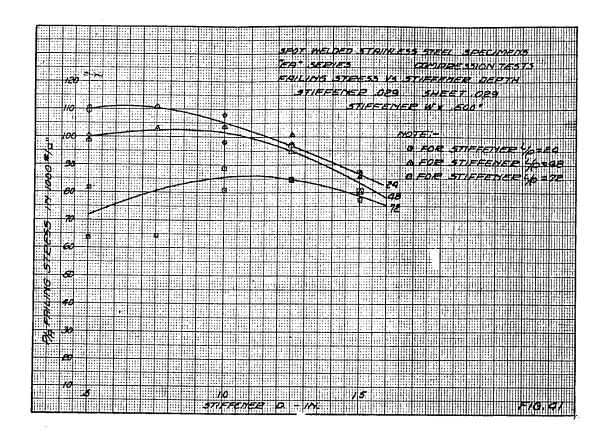


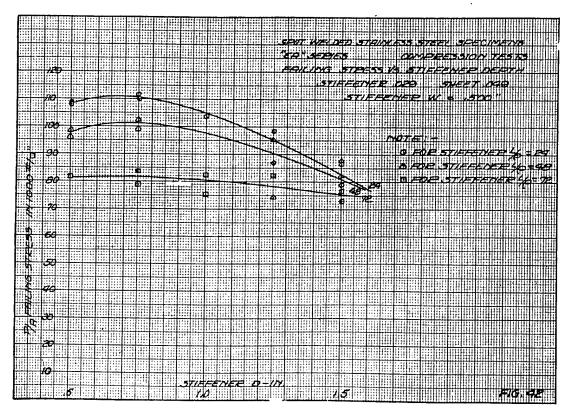


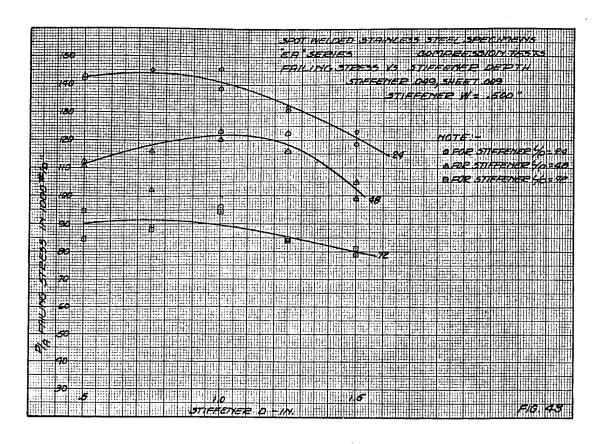
)

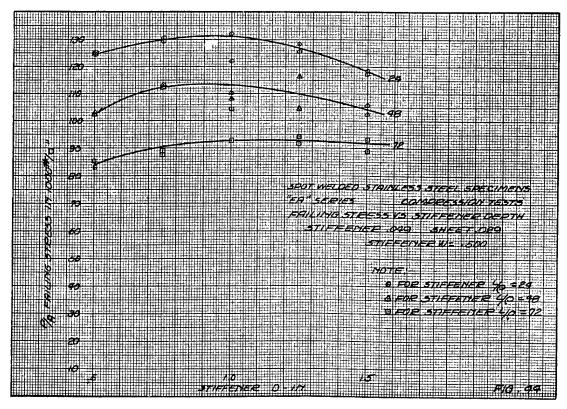


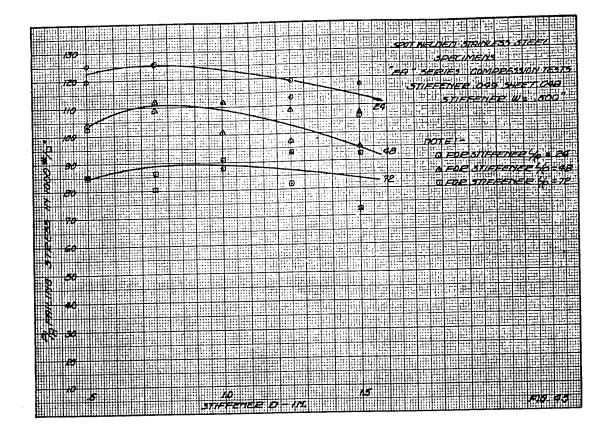


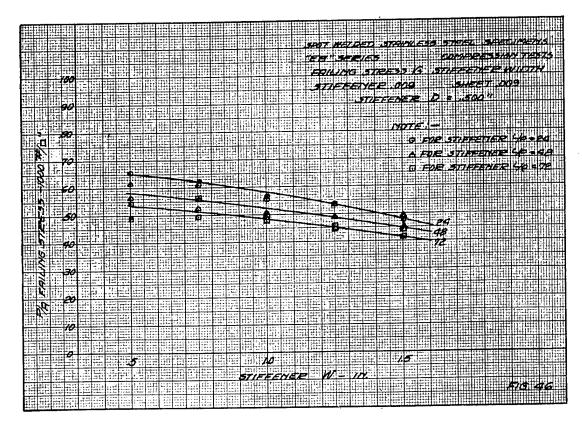


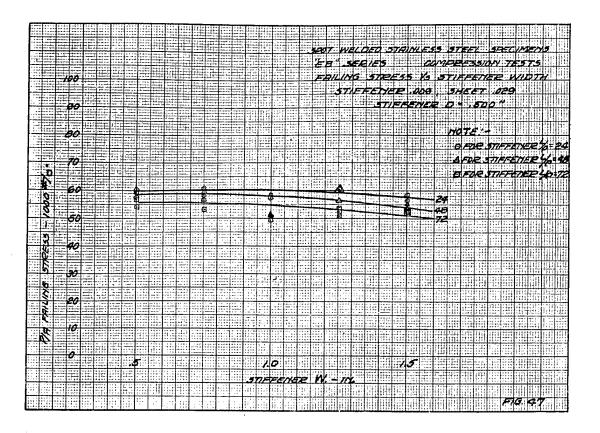


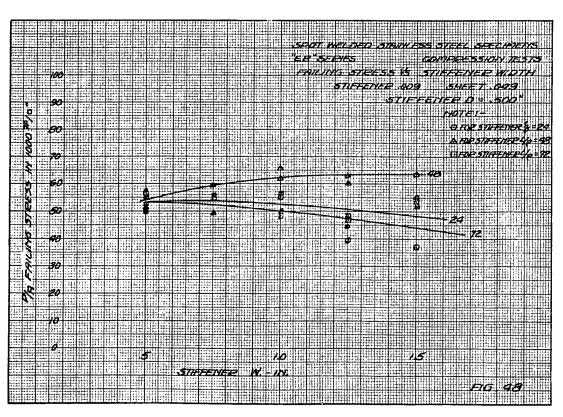


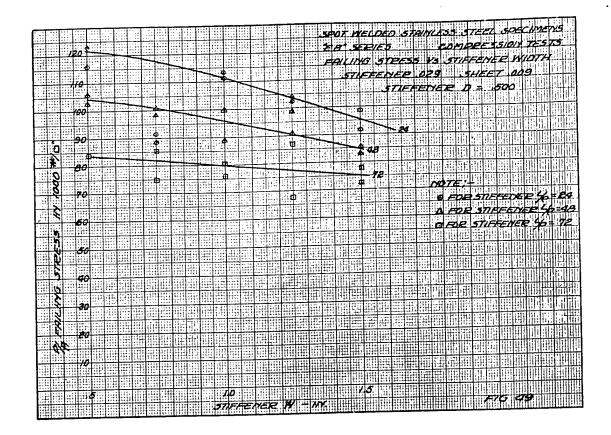


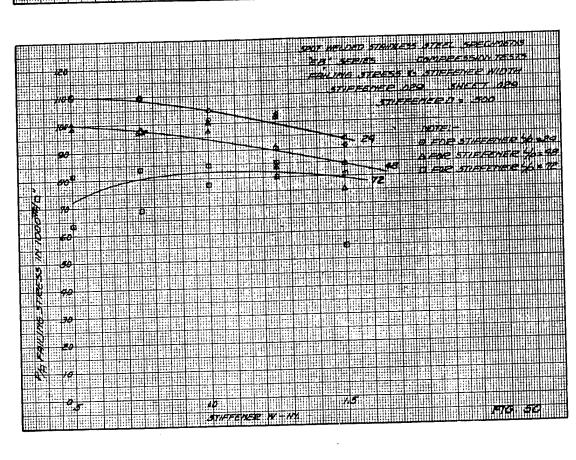


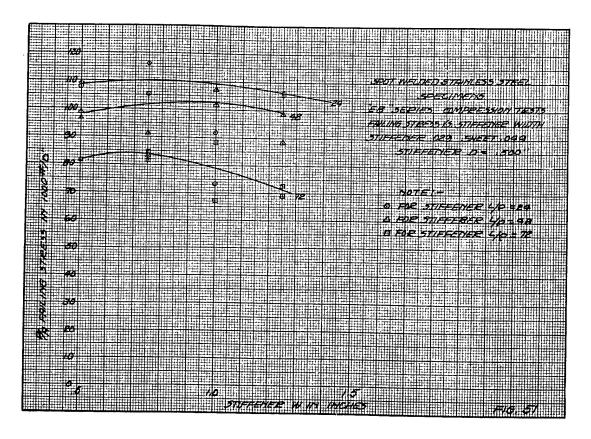


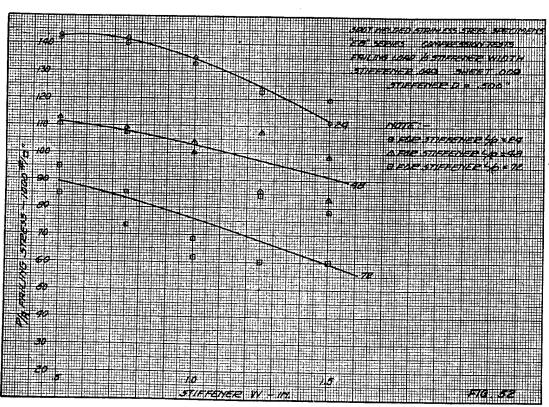


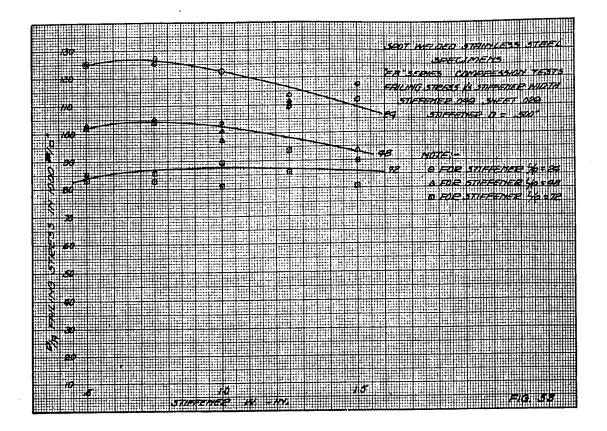


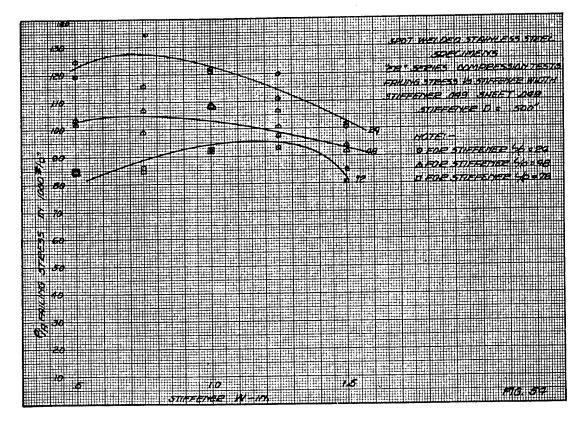


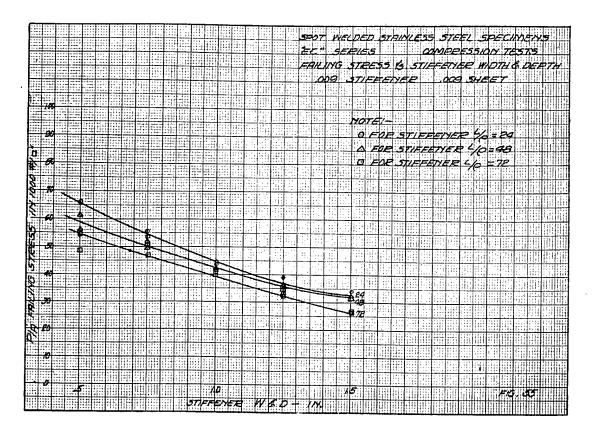


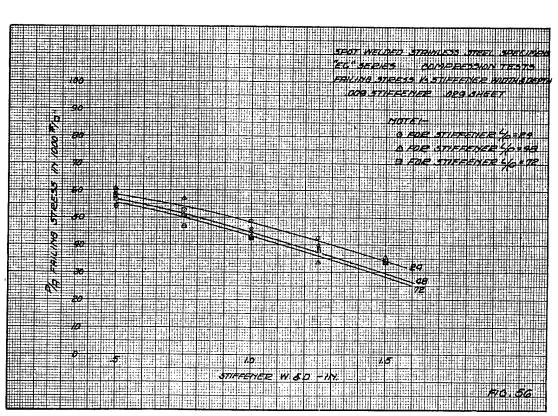


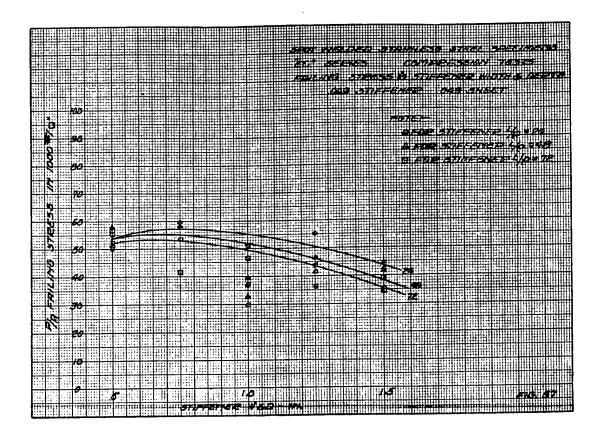


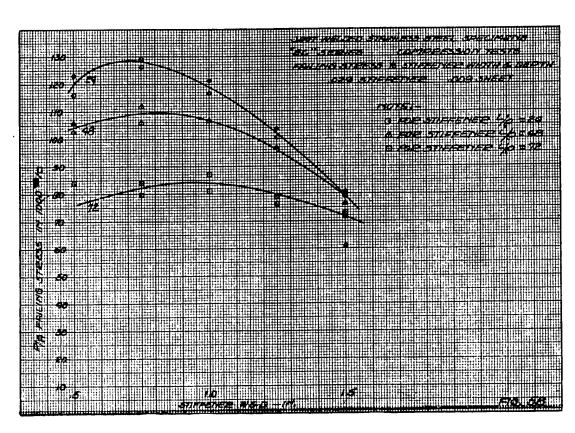


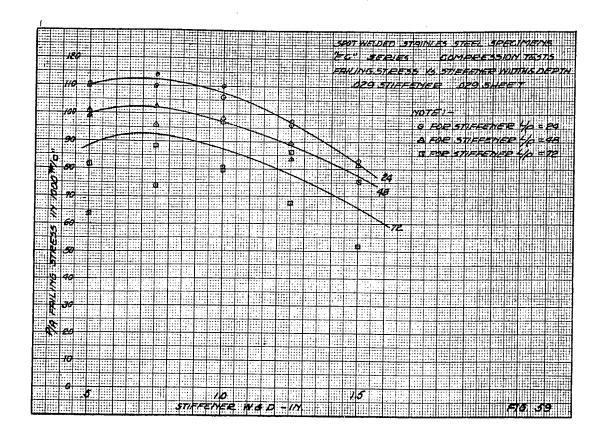


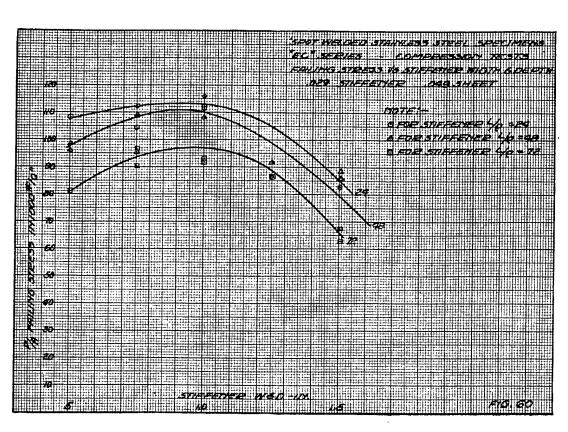


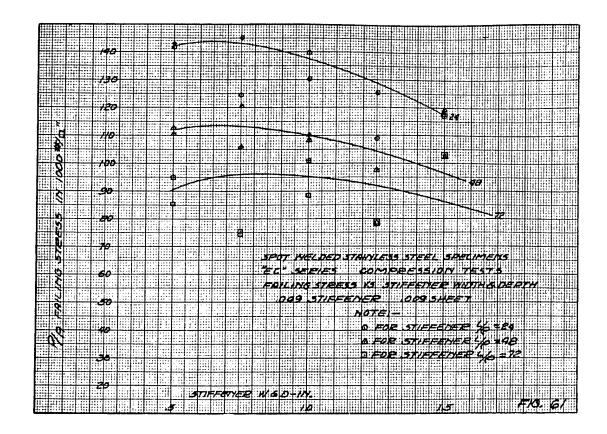


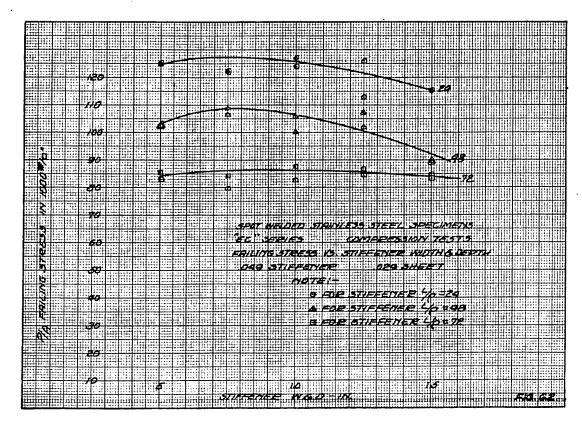


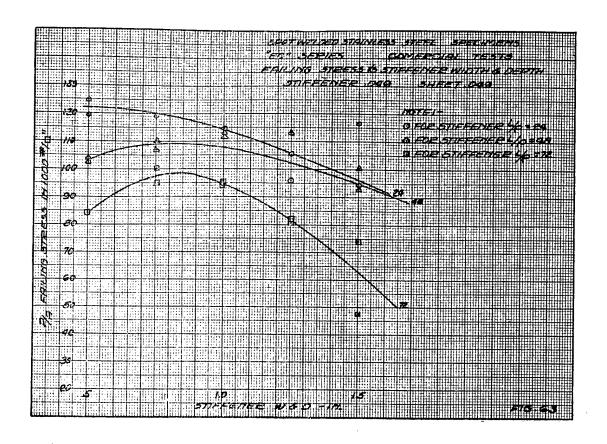


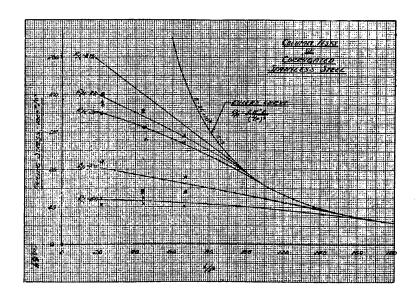


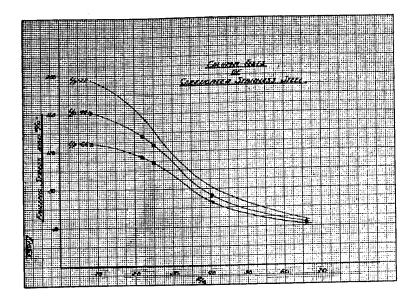


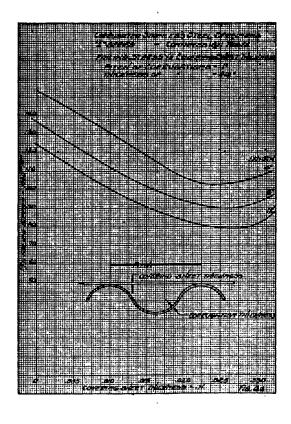


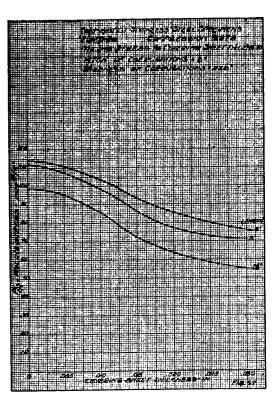


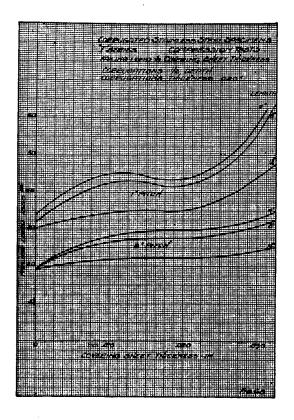


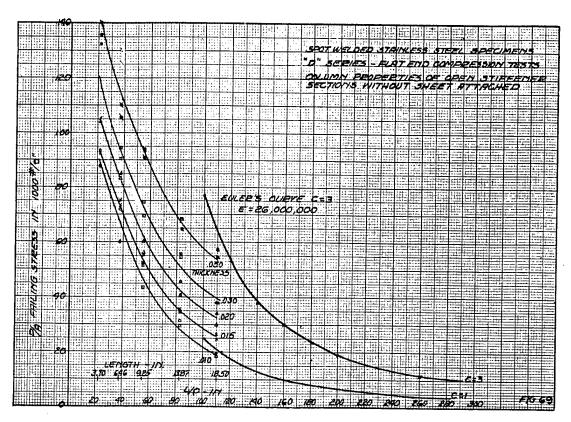


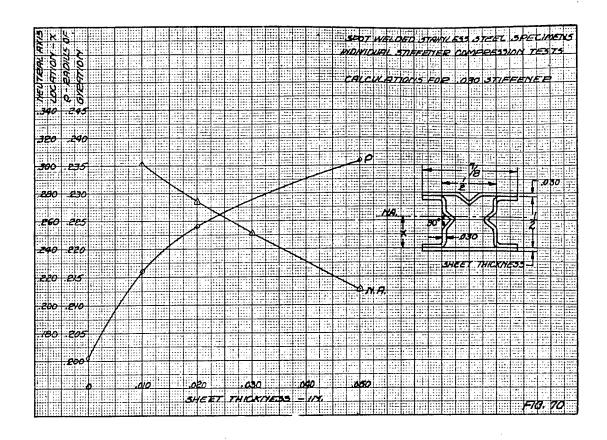


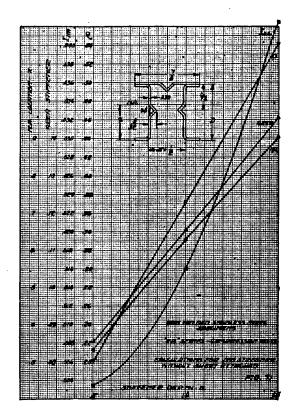


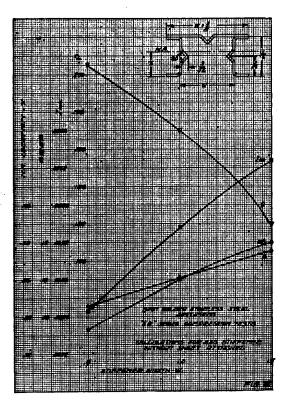


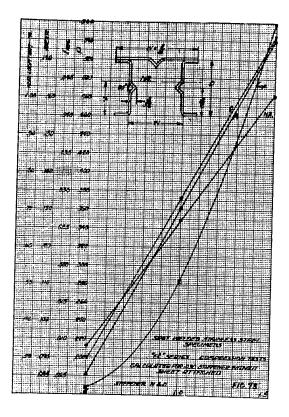












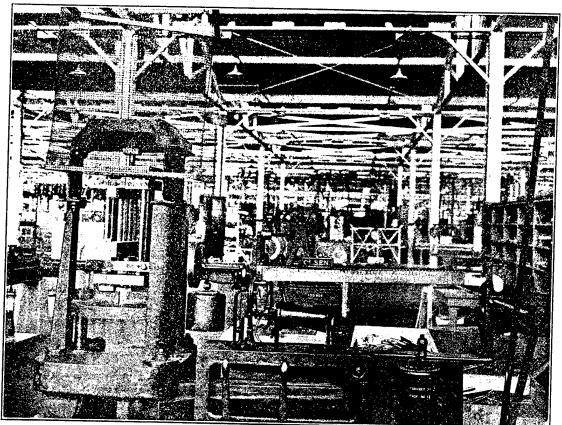


FIGURE 74.—Compression Test Set-up with Short Screws in Jig.



FIGURE 75.-Compression Test Set-up with Long Screws in Jig.

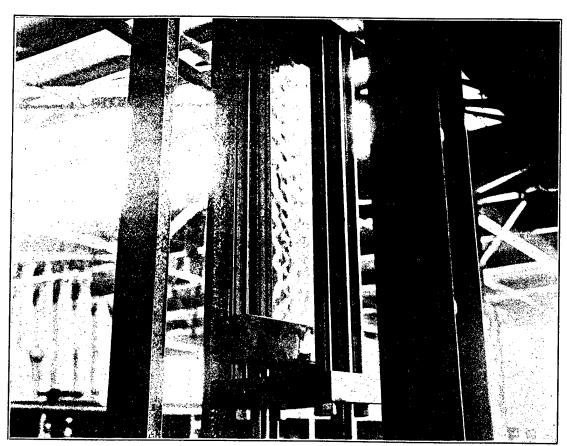
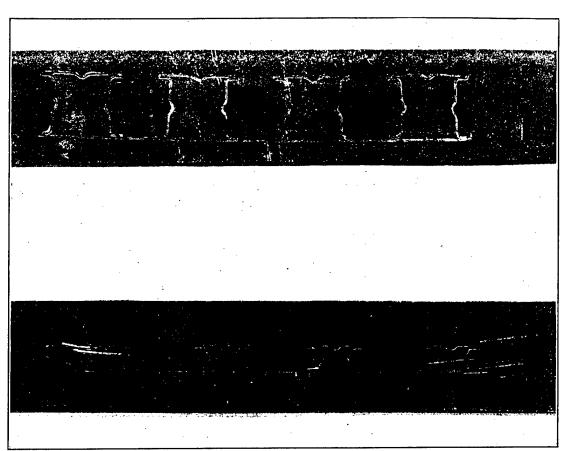


FIGURE 76.—Close-up View of Set-up of Specimen Using Long Screws in Jig.



 ${\tt Figure~77.-Typical~Impression~of~Specimens~Left~on~Dural~Seating~Strips.}$ 

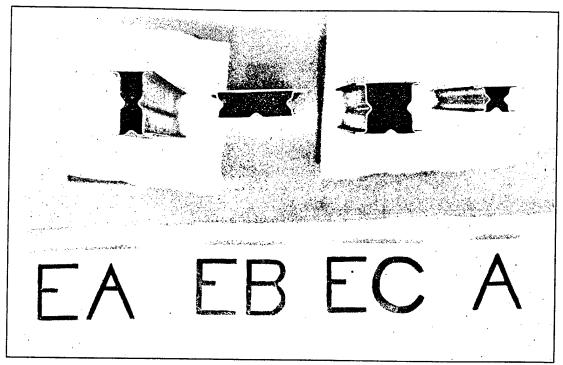


FIGURE 78.—Typical EA, EB, EC, and A Series Stiffener Sections.

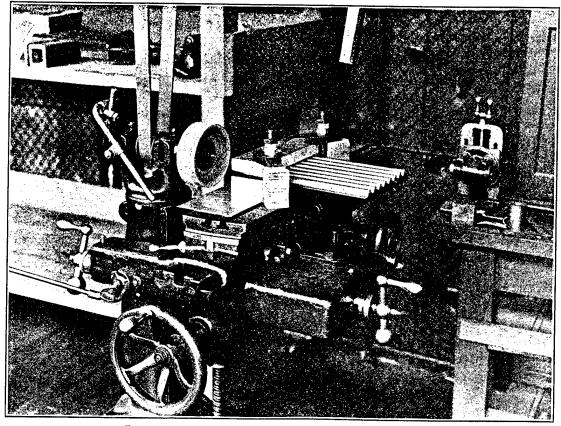


Figure 79.—Set-up Used for Regrinding the Ends on the I Series Specimens.

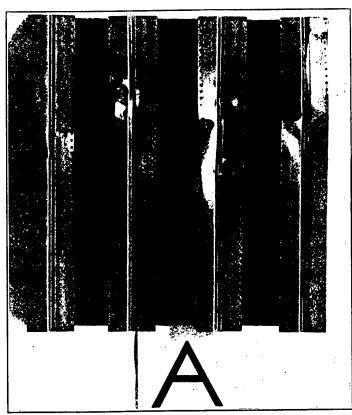


FIGURE 80.-Type A Failure.

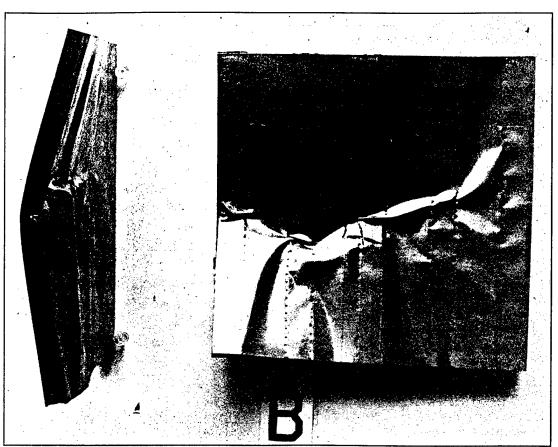


FIGURE 81.—Type B Failure.

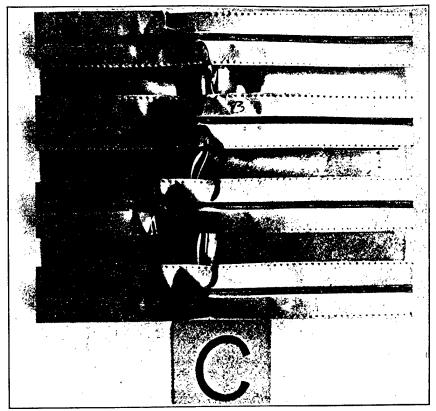


FIGURE 82.-Type C Failure.

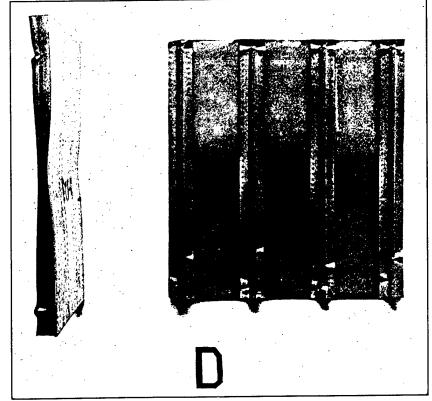


FIGURE 83.—Type D Failure.

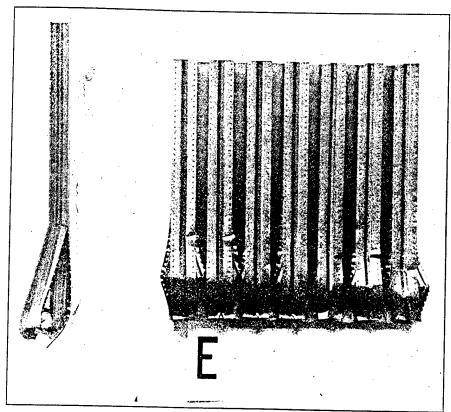


FIGURE 84. -Type E Failure.

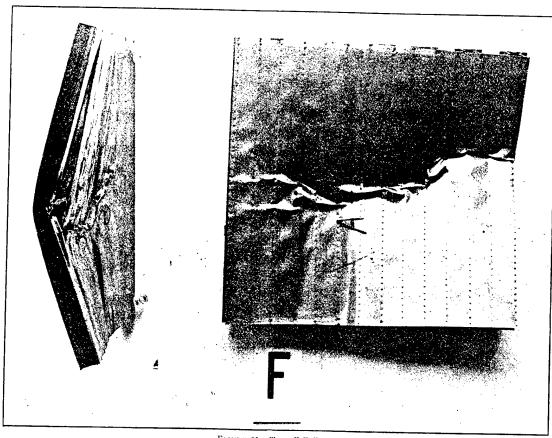


FIGURE 85. - Type F Failure.

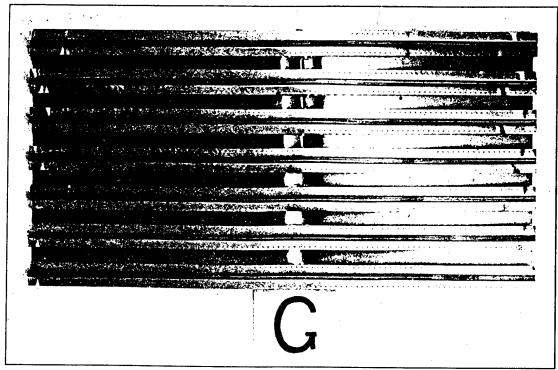


FIGURE 86. -Type G Failure.



FIGURE 87.—Type H Failure.

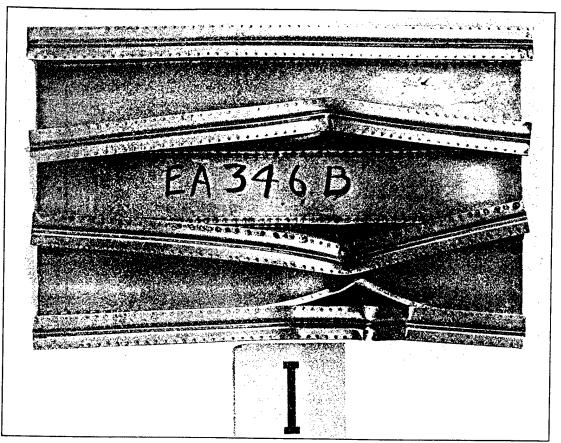


FIGURE 88.-Type I Failure.

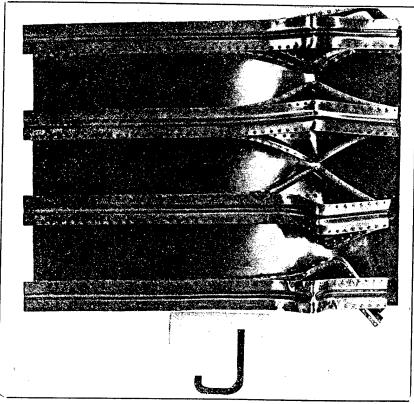


FIGURE 89.—Type J Failure.

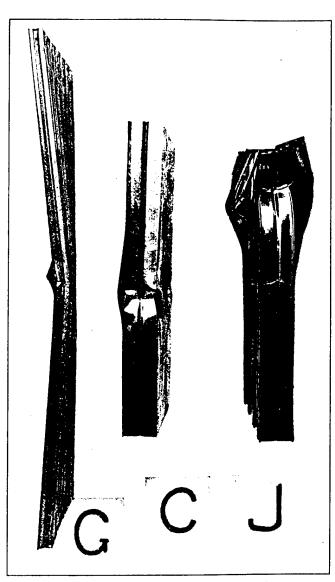


FIGURE 90.-Side View of Type C, G, and J Failures.

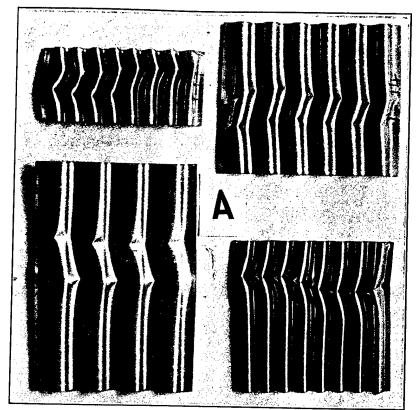


FIGURE 91.—Buckling Failure of Corrugated Sheet.

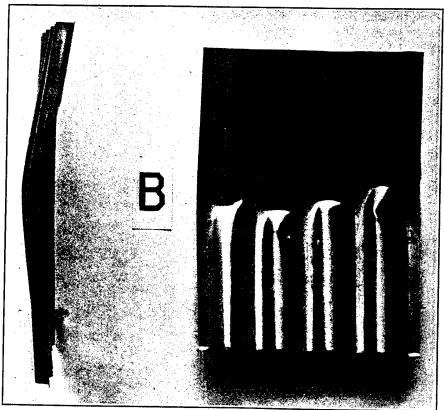


FIGURE 92.—Column Failure of Corrugated Sheet.

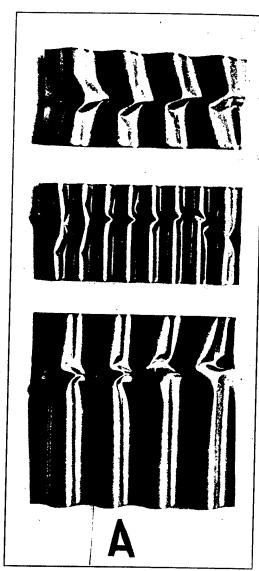
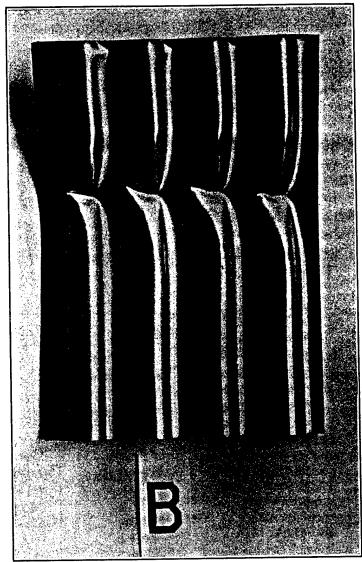


FIGURE 93.—Buckling of Corrugated Sheet with Flat Sheet Attached.



 $\textbf{F}_{\textbf{IGURE}} \ \textbf{94.--} \textbf{Column} \ \textbf{Failure} \ \textbf{of} \ \textbf{Corrugated} \ \textbf{Sheet} \ \textbf{with} \ \textbf{Flat} \ \textbf{Sheet} \ \textbf{Attached}.$